



Traffic & Transport Assessment

Project:

Proposed Strategic
Housing Development,
Kenelm, Deer Park,
Howth, Co. Dublin

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Contact Information

Barrett Mahony Consulting Engineers Ltd

52-54 Lower Sandwith Street, Dublin 2
D02 WR26

Telephone: 01 677 3200

Email: bmce@bmce.ie

www.bmce.ie

BM BARRETT MAHONY
CONSULTING ENGINEERS
CIVIL & STRUCTURAL

TABLE OF CONTENTS

1.0	INTRODUCTION	5
1.1	GENERAL DESCRIPTION	5
1.2	INITIAL CONSULTATION WITH FINGAL COUNTY COUNCIL	5
1.3	PURPOSE OF TRAFFIC AND TRANSPORT ASSESSMENT.....	5
1.4	METHODOLOGY UTILISED WITHIN TRAFFIC AND TRANSPORT ASSESSMENT	6
1.5	SITE ACCESS TO ROAD NETWORK.....	7
1.6	SCOPE OF THE REPORT	8
2.0	RECEIVING ENVIRONMENT.....	10
2.1	LOCATION OF PROPOSED DEVELOPMENT.....	10
2.2	CYCLING AND PUBLIC TRANSPORT INFRASTRUCTURE	10
2.3	OVERALL COMMENT ON RECEIVING ENVIRONMENT	15
3.0	PLANNING DEVELOPMENT AND CONTROL FRAMEWORK.....	16
3.1	FINGAL COUNTY DEVELOPMENT PLAN 2017 – 2023.....	16
3.2	CAR AND CYCLE PARKING REQUIREMENTS AS PER FINGAL COUNTY DEVELOPMENT PLAN 2017 - 2023 16	
3.2.1	<i>INTRODUCTION.....</i>	<i>16</i>
3.2.2	<i>CAR AND CYCLE PARKING: PERMITTED VS MAXIMUM REQUIREMENTS OF FCC.....</i>	<i>16</i>
3.2.3	<i>CAR PARKING REQUIREMENTS FOR THE RESIDENTIAL COMPONENT BASED ON NEW APARTMENT GUIDELINES</i>	<i>17</i>
3.3	COMMENT ON PARKING PROVISION.....	18
4.0	TRIP GENERATION, DISTRIBUTION AND ASSIGNMENT FOR PROPOSED DEVELOPMENT	19
4.1	INTRODUCTION.....	19
4.2	TRIPS GENERATED BY CANDIDATE SITE	19
4.3	TRIPS GENERATED BY NEARBY PLANNED DEVELOPMENTS (TECHCRETE PLUS BALSCADDEN)	19
4.4	RELATIVE IMPACT OF PROPOSED DEVELOPMENT RELATIVE TO PROPOSED ADJACENT SCHEMES	20
4.5	DISTRIBUTION OF GENERATED FLOWS FROM PROPOSED AND PLANNED DEVELOPMENTS	20
4.5.1	<i>PROPOSED DEVELOPMENT.....</i>	<i>20</i>
4.5.2	<i>PLANNED DEVELOPMENTS AT TECHCRETE AND BALSCADDEN ROAD.....</i>	<i>20</i>
4.6	TRIP ASSIGNMENT	21
4.7	IMPACT OF GENERATED FLOWS ON CRITICAL JUNCTIONS	21
5.0	TRAFFIC IMPACT ASSESSMENT OF 5 NO. CRITICAL JUNCTIONS IN PROXIMITY TO THE SUBJECT SITE .	22
5.1	INTRODUCTION.....	22
5.2	ANALYSIS JUNCTION OF SUTTON CROSSROADS SIGNALISED JUNCTION	23
5.2.1	<i>GEOMETRIC PARAMETERS.....</i>	<i>23</i>
5.2.2	<i>ANALYSIS OF AM AND PM PEAK HOUR FLOWS FOR THE 7 NO. SCENARIOS</i>	<i>23</i>
5.3	ANALYSIS OF DEVELOPMENT ENTRANCE PRIORITY JUNCTION	25
5.3.1	<i>GEOMETRIC PARAMETERS.....</i>	<i>25</i>
5.3.2	<i>ANALYSIS OF AM AND PM PEAK HOUR FLOWS FOR THE 2 NO. SCENARIOS</i>	<i>25</i>
5.4	ANALYSIS OF HOWTH ROAD / CHURCH ROAD PRIORITY JUNCTION	26
5.4.1	<i>GEOMETRIC PARAMETERS.....</i>	<i>26</i>
5.4.2	<i>ANALYSIS OF AM AND PM PEAK HOUR FLOWS FOR THE 2 NO. SCENARIOS</i>	<i>26</i>
5.5	ANALYSIS OF HOWTH ROAD / OFFINGTON PARK PRIORITY JUNCTION	27
5.5.1	<i>GEOMETRIC PARAMETERS.....</i>	<i>27</i>
5.5.2	<i>ANALYSIS OF AM AND PM PEAK HOUR FLOWS FOR THE 2 NO. SCENARIOS</i>	<i>27</i>
5.6	ANALYSIS OF HARBOUR ROAD / CHURCH STREET PRIORITY JUNCTION.....	27
5.6.1	<i>GEOMETRIC PARAMETERS.....</i>	<i>27</i>
5.6.2	<i>ANALYSIS OF AM AND PM PEAK HOUR FLOWS FOR THE 2 NO. SCENARIOS</i>	<i>27</i>

6.0	SUMMARY COMMENTS ON TRAFFIC IMPACT OF PROPOSED DEVELOPMENT	28
6.1	SUMMARY OF ANALYSIS.....	28
6.2	CONCLUSIONS FROM ANALYSIS	28
6.3	MITIGATION.....	29

APPENDICES

APPENDIX 1 – SITE LAYOUT PLAN

APPENDIX 2 – TRAFFIC SURVEYS OCTOBER 2019

APPENDIX 3 – FLOW DIAGRAMS

APPENDIX 4 – DETAILED JUNCTION ANALYSIS

1.0 INTRODUCTION

1.1 GENERAL DESCRIPTION

Barrett Mahony Consulting Engineers have been commissioned to provide a Traffic and Transport Assessment (TTA) for the proposed residential development at Kenelm, Deer Park, Howth, Co. Dublin. This TTA is to be read in conjunction with the Residential Travel Plan, document 19.196-IR-03, prepared by BMCE which accompanies this SHD application.

The proposed development will consist of 162 No. apartments, with 132 No. car parking spaces (including 6 No. disabled, 4 no. car sharing & 13 EV parking) and 355 No. cycle parking spaces (with 325 no. at basement level for long stay parking and 30 no. at ground floor level for short stay visitors).

The above provisions equate to 0.81 No. car parking spaces per unit and 2.19 No. cycle parking spaces per unit.

1.2 INITIAL CONSULTATION WITH FINGAL COUNTY COUNCIL

In order to determine the scope of the required TTA, Dr Martin Rogers contacted Mr Niall Thornton, Senior Executive Engineer, Fingal County Council (FCC) in early October 2019.

Dr Rogers suggested that, in line with previous applications in the Howth area, the following junctions should be analysed:

- Sutton Cross Signalised junction
- Howth Road / Church Road priority junction
- Howth Road / Offington Park priority junction
- Harbour Road / Church Street priority junction

On 7th October 2019, Mr Thornton replied stating that the junctions identified were in order. Specifically, in relation to the critical junction at Sutton Cross; he stated that the report should reference it, identifying its impact relative to the proposed adjacent developments at Techcrete and Balscadden.

Therefore, in order to facilitate compliance with this request, traffic surveys at the above 4 No. junctions were carried out on Tuesday 22nd October 2019.

The traffic assessment within this report will analyse the existing flows on the major adjacent intersections, detail the existing level of operational efficiency at these locations, and will also assess the impact that the flows predicted to be generated by both the proposed and adjacent developments will have on these operational efficiencies.

1.3 PURPOSE OF TRAFFIC AND TRANSPORT ASSESSMENT

The purpose of this Traffic and Transport Assessment is to assess the current operational efficiency of the existing transport environment and provide details of the assessment undertaken, to identify the level of transport impact resulting from the proposed residential development. The scope of the assessment covers both transport and related sustainability issues, including means of vehicular access, pedestrian, cyclist and local public transport connections. The principal objective of the report is to quantify any level of impact across the local road network and subsequently ascertain both the existing and future operational performance of the local road network.

The traffic assessment within this report will analyse the existing flows on the major adjacent intersections, detail the existing level of operational efficiency at these locations, and will also assess the impact that the flows predicted to be generated by both the proposed and adjacent developments will have on these operational efficiencies.

This assessment will also take account of the permitted adjacent SHD development at the former Techrete site and will consider the potential flows associated with the former permitted but now quashed Balscadden Road

site. We deem that taking account of traffic flows from both these sites in addition to the subject site will result in a very robust analysis.

The Claremont site (TA06F.306102), former Techcrete site, has permission for a mixed-use development including 512 no. units, 439 car parking spaces of which 80 are designated to serve the commercial development. The Balscadden development (TA06F.301722), former Bailey Court, received permission for a mixed-use development including 164 no. units and 120 car parking spaces. The decision was subsequently quashed by the High Court. It is noted that this Traffic and Transport Assessment takes account of this development as a conservative approach to the assessment. The site is zoned for development and it is assumed that a similar development would be progressed in the future.

The analysis within this report is undertaken on the basis of 1.4% annual growth in network traffic over the period 2019 to 2030 period, decreasing to 0.4% in the 2030 to 2037 period, consistent with the 'medium growth' assumption for the four planning authorities within the Dublin city area as detailed within the 2016 Transport Infrastructure Ireland document 'Project Appraisal Guidelines for National Roads Unit 5.3 – Travel Demand Projections', PE-PAG-02017 October 2016.

1.4 METHODOLOGY UTILISED WITHIN TRAFFIC AND TRANSPORT ASSESSMENT

This report was developed with guidance from the documents listed below;

- 'Traffic and Transport Assessment Guidelines' (May 2014) National Road Authority;
- 'Traffic Management Guidelines' Dublin Transportation Office & Department of the Environment and Local Government (May 2003);
- 'Guidelines for Traffic Impact Assessments' The Institution of Highways and Transportation; and
- Fingal County Council Development Plan 2017-2023.

The methodology utilised can be divided into the following 5 No. phases, in compliance with the Traffic and Transport Assessment Guidelines referenced above:

Audit of existing network

The report establishes the existing level of accessibility at present pertaining to the subject site in terms of the level of access available by walking, cycling and public transport.

Completion of Traffic Counts

The report details junction traffic counts undertaken at the locations agreed with FCC and analysed in order to assess existing operating efficiencies in the vicinity of the proposed development.

Estimation of Trip Generation Volumes

A trip generation exercise has been carried out to establish an estimate for the level of vehicle trips generated by the proposed residential development.

Distribution of Generated Trips

Based upon both the existing observed flow patterns in the local road network at the identified relevant junctions, the trips predicted to be generated by the proposed development are distributed / assigned onto the local road network.

Network Analysis detailing Impact of Generated Volumes

Junction analysis models are utilised to analyse the impact of the estimated trip generation volumes on the operational efficiency of the junctions selected for detailed analysis.

This analysis is undertaken for both the year of opening of the proposed development and the 'design year' fifteen years thereafter.

This methodology is consistent with the following sections required within a basic Traffic and Transport Assessment for compliance with the 2014 TTA Guidelines:

- Introduction / Existing conditions
- Extent of proposed development (including existing and future public transport and walking / cycling facilities)

- Vehicular Trip Generation
- Vehicular Trip Distribution / Assignment to network
- Impact on road network of trips generated by proposed development

1.5 SITE ACCESS TO ROAD NETWORK

The site is located on Howth Road, adjacent to the entrance to Howth Castle and DeerPark Golf Club.

A site location map is contained within Figure 1-1 below. A site layout is contained within Figure 1-2.

Appendix 1 contains a ground floor plan of the proposed development, including details of the vehicular access onto Howth Road.

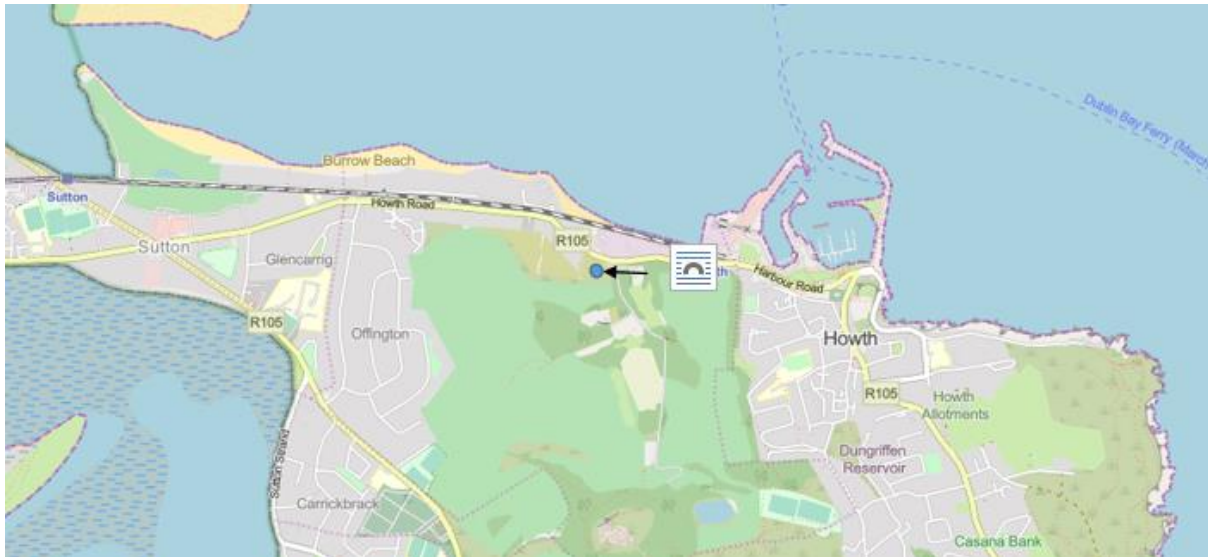


Figure 1-1: Site location map (indicative location in blue)



Figure 1-2: Site layout

Sightlines are good in both directions for exiting drivers, complying with the requirements of DMURS i.e. both forward visibility stopping sight distances (SSD) of 59m for standard roads and 65m for bus routes are available at a 2.4m set-back at the site access for Howth Road which is a 60 km/h speed zone.

A site location map, indicating the location of the 4 No. traffic surveys is contained within Figure 1-3.

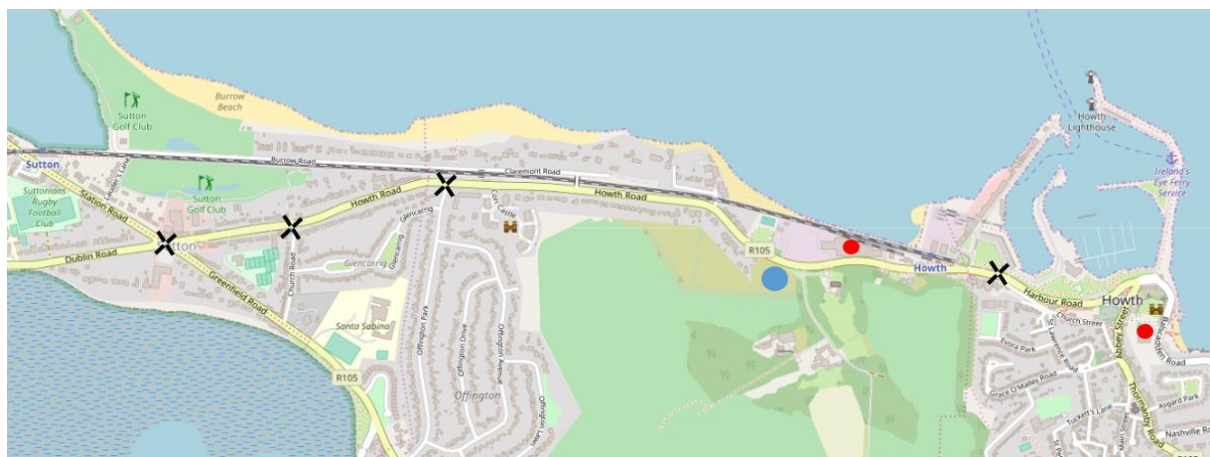


Figure 1-3: Location of 4 No. surveys with site shown in Blue and Techcrete and Balscadden Sites shown in Red.

1.6 SCOPE OF THE REPORT

This report is structured in order to address in detail the 5 No. stages of stated methodology as detailed within section 1.4 above.

Section 2 provides details of the receiving environment, detailing existing conditions pertaining at the site of the proposed development and the surrounding local road network.

Section 3 details the planning development and control framework associated with the proposed site, including relevant FCC planning policy and FCC development control policy regarding parking for bicycles and cars.

Section 4 details the trip generation, trip distribution process, assigning the estimated flows to the junctions chosen for analysis.

Section 5 details an analysis of the traffic impact of the proposed and committed development on the development entrance and nearby critical junctions for the existing situation, the estimated year of opening, and within the design year, fifteen years thereafter.

Section 6 makes some concluding comments regarding the sustainability of the proposed project in transport impact terms.

2.0 RECEIVING ENVIRONMENT

2.1 LOCATION OF PROPOSED DEVELOPMENT

The general location of the subject site in relation to the surrounding road network is illustrated in Figure 1.1 above which details the extent of the subject site boundary and neighbouring lands.

The site is located on the southern side of Howth Road, immediately adjacent to the Howth Castle entrance, approximately 1 km east of Howth village.

Given that the proposed development is residential, peak flows will typically occur on weekdays, with peak flows typically occurring between 7am and 9am in the morning and between 4pm and 6pm in the evening.

Accordingly, traffic surveys were carried out on Tuesday 22nd October 2019 at the 4 No. junctions.

The raw data from these surveys is contained within Appendix 2.

The surveys were carried out over a 12-hour period between 0700 and 1900 in order to ascertain the peak hour flows for all traffic movements at the 4 No. junctions.

The surveys indicated that the weekday morning peak occurred between 0800 and 1000 with the evening peak occurring between 1600 and 1800 – these were observed to be the timeframes during which the junctions were most heavily loaded. The following analysis is based on these peak periods.

On the basis of the results of both the surveys and assumptions regarding when peak flows from the generated traffic will occur, the morning peak hour has been taken as 0800 to 0900, with the evening peak taken to occur between 1700 and 1800.

The survey data is detailed for the morning and evening peak hours in Diagrams 1 and 2 within Appendix 3.

Table 2-1 indicates the total flows incident on all 4 No. junctions during the morning and evening peaks, and compares the values obtained with the results from January 2019 surveys completed for an adjacent permitted development at the Claremont site:

		JANUARY 2019		OCTOBER 2019		AVERAGE +/-
		8 to 9	5 to 6	8 to 9	5 to 6	
		AM	PM	AM	PM	
Sutton Crossroads	site 1	1949	1694	1944	1602	-2.8
Church Road / Howth Road	site 2	983	824	1055	972	+12.6
Offington Park / Howth Road	site 3	898	797	945	917	+10.1
Harbour Road / Church Street	site 4	658	712	730	719	+6.0

Table 2-1: Comparison of January 2019 and October 2019 surveys at 4 No. critical junctions

Thus, the two surveys are virtually identical at the Sutton Crossroads location, with average increases within the three internal junctions of 9%.

2.2 CYCLING AND PUBLIC TRANSPORT INFRASTRUCTURE

EXISTING CYCLE NETWORK

The “Cycle Network Plan for the Greater Dublin” area has produced an overall plan for providing safe cycle routes both within the city and in the suburbs.

Figure 2-1 contains the map of existing cycle facilities for the area close to the subject site as detailed within the GDA Cycle Plan.



Legend:

- B1 - Bus Lane (no cycle lane)
- C1 - Cycle Track - separated from road
- C2 - Cycle Track - immediately adjacent
- C3 - Cycle Lane (even within Bus Lane)
- G1 - Cycle Trail or Greenway
- S2 - Shared Walking & Cycling
- Study Area
- County Council Boundaries
- Greenline Tram Stops
- Redline Tram Stops
- Stations

Figure 2-1: Existing cycle facilities close to the proposed development (GDA cycle plan)

It can be seen that, at present, the major cycle lane is along the bus corridor on the Howth Road, linking the site to Sutton Cross and onwards towards the city centre.

PROPOSED CYCLE NETWORK

Figure 2-2 details the network improvements proposed within the GDA cycle plan.



Figure 2-2: Proposed cycle facilities close to the proposed development (GDA cycle plan)

A secondary cycle route is planned along Howth Road and Carrickbrack Road, which will connect the subject site to all parts of Howth, southwards towards the city centre and north-westwards towards Portmarnock, Malahide and Swords.

In addition, the proposed East Coast Greenway will run on the northern edge of the site, connecting Howth to the greenway network in the Greater Dublin area.

Figure 3-3 contains a drawing of the Dublin Greenway network map, indicating the extent of the east Coast Greenway.



Figure 2-3: Dublin Greenway Map, including route of East Coast Greenway

PUBLIC TRANSPORT - BUS

The Dublin Bus services in the area provide direct linkage to the city, the Route 31/a along Howth Road towards the city centre, and the 31b Route along Carrickbrack Road towards the city centre.

The frequency of each route during the morning peak is detailed within Table 2-1.

Route	Origin	Destination	Frequency (08:00 – 09:00)
Route 31/a	Howth Road / Carrickbrack Road	Talbot Street	2 per hour
Route 31b	Carrickbrack Road	Talbot Street	1 per hour

Table 2-2: Dublin Bus Route Frequencies

Future bus plans involve the “Dublin Area Bus Network Redesign” (Bus Connects) which will overhaul the current bus system in the Dublin region by developing new bus corridors, new bus routes, increasing services and new buses.

Figures 2-4 provides an overview of the provision of services which will upgrade the current Dublin bus service.

Figure 2-5 details the existing bus routes serving the subject site, emphasising the proximity of the routes 31 and 31a to the proposed development.

Given the existence of the DART service, Bus Connects proposes the N6 orbital route across the north side of Howth, opening up a new service to DCU while maintaining a good connection to the rail or the D spine for travel to the city centre.

On the southern and western sides of Howth, where demand is relatively low, local routes 290 and 291 will operate an hourly service, providing direct service to Sutton and Clongriffin DART Stations.



Figure 2-4: Bus Connects Extract – proposed network

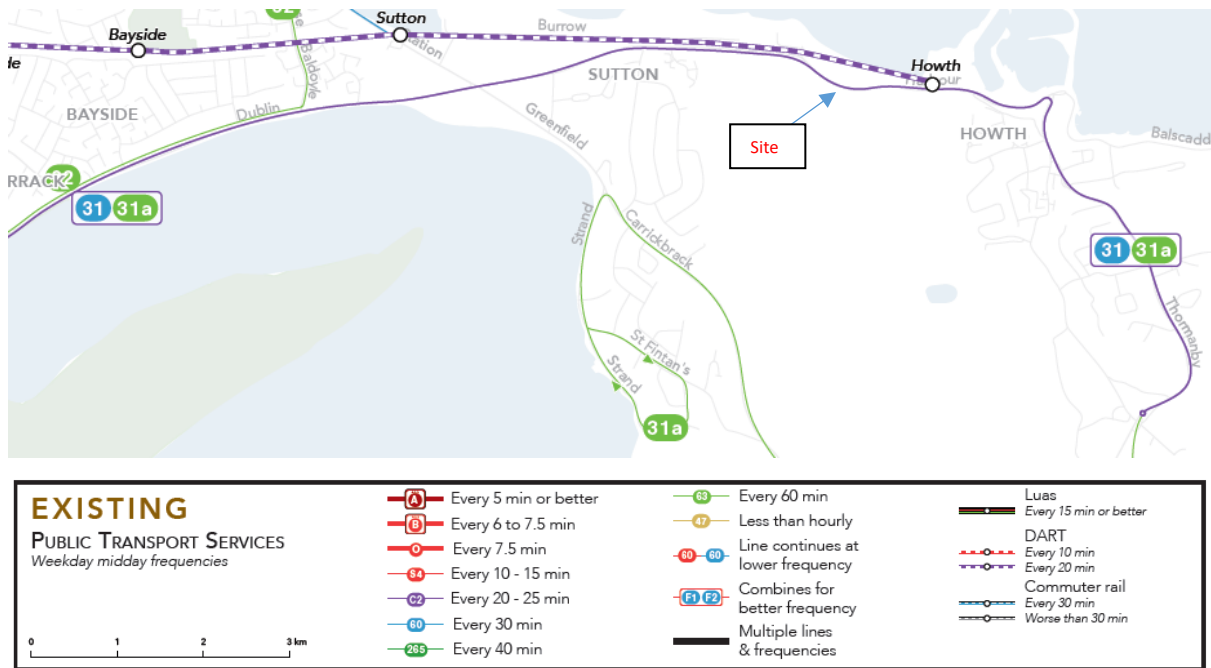


Figure 2-5: Existing bus services (31 31a) close to subject site

DART SERVICE

The DART extends along the coastline of the South Dublin area, extending from the centre of town to Ballsbridge, Sandymount, Merrion, Booterstown, Blackrock, Monkstown, Dun Laoghaire, Dalkey, Ballybrack, Shankill, Bray and Greystones, and along the coastline of the north Dublin area extending from the town centre to Clontarf, Sutton, Howth and Malahide.

The Howth DART Station is within 450 metres (5 minutes' walk) of the subject site.

The DART operates a service to the city centre every 10 to 15 minutes during the morning peak time.

Figure 2-6 contains diagrammatic representations of the DART system serving the site and its connectivity to the regional / national rail network.

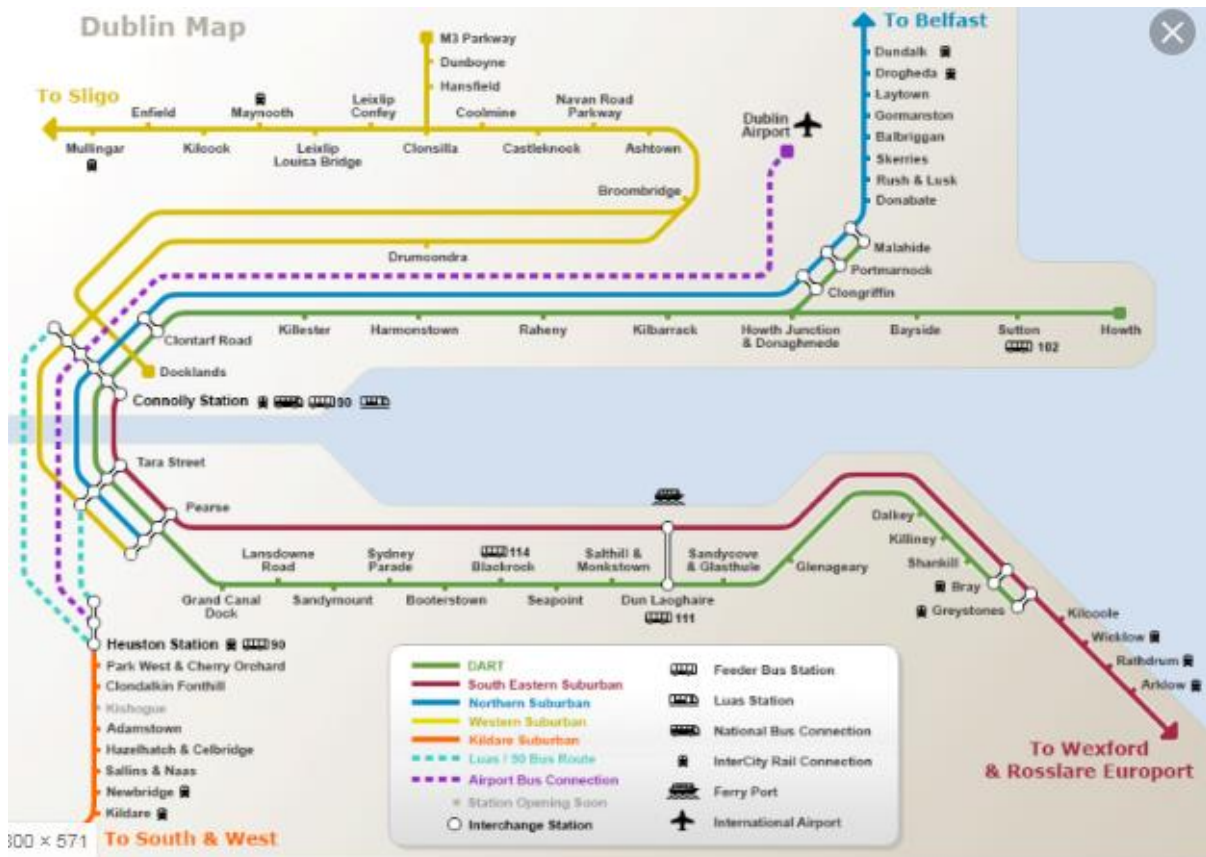


Figure 2-6: Diagrammatic representation of DART line and its connectivity to regional / national rail network

2.3 OVERALL COMMENT ON RECEIVING ENVIRONMENT

In conclusion, it must be stated that the receiving environment fits very positively with the proposed residential development. The existence of significant public transport facilities – the DART Line within 450 metres of the subject site, in addition to the Route 31a and 31b buses running along the Howth Road, excellent pedestrian facilities, linking the site to nearby suburbs, plus the prospect of planned upgrades to the cycling and bus network will result in a development which will be served by significant sustainable transport modes both for local trips and trips to nearby urban / suburban centres.

3.0 PLANNING DEVELOPMENT AND CONTROL FRAMEWORK

3.1 FINGAL COUNTY DEVELOPMENT PLAN 2017 – 2023

This document provides the broad framework for the Fingal administrative area. In the context of the proposed residential development, the following transport and mobility objectives are relevant:

- Promote and facilitate movement to, from, and within the County of Fingal, by integrating land use with a high quality, sustainable transport system that prioritises walking, cycling and public transport.
- Provide an appropriate level of safe road infrastructure and traffic management, in particular to support commercial and industrial activity and new development.
- Work with all relevant stakeholders to seek a reduction in greenhouse gas emissions from transport.

The existing and proposed transport infrastructure associated with the proposed development, as detailed above within section 2, is seen as totally consistent with the achievement of the above objectives for sustainable transport provision for future commuters.

3.2 CAR AND CYCLE PARKING REQUIREMENTS AS PER FINGAL COUNTY DEVELOPMENT PLAN 2017 - 2023

3.2.1 INTRODUCTION

This section of the report will detail the car and cycle parking requirements for the proposed development based on the Fingal Development Plan 2017-2023 and the Sustainable Urban Housing: Design Standards for New Apartments (Guidelines for Planning Authorities) was published by the Department of Housing, Planning and Local Government in March 2018.

The proposed car and cycle parking provision on site will then be detailed, highlighting, in particular, the intended low level of provision in relation to car parking for the residential component of the proposed development.

It will be justified that the proposed residential parking provision is entirely sustainable given the current car ownership and modal splits for the journey to work / college for existing residents living close to the subject site.

This reduced level of provision is also seen as being completely consistent with the mobility targets for Dublin city as detailed within the Dublin City Transport Plan and also consistent both with minimising the traffic impact of nearby already congested junctions, particularly Sutton Cross, (as detailed within the accompanying traffic impact assessment) and with maximising patronage of the extensive public transport and soft mode options (as detailed within this mobility plan).

3.2.2 CAR AND CYCLE PARKING: PERMITTED VS MAXIMUM REQUIREMENTS OF FCC

Table 3-1 below details the maximum car and bicycle parking standards for Fingal County Council based on the rates contained within their 2017 - 2023 Development Plan Written Statement for the proposed residential development:

Development type	Area / units	Maximum car parking standards	Parking required
Apartments 1-bed	29 No.	1.0 per unit plus 0.2 per unit visitor spaces	35
Apartments 2-bed	104 No.	1.5 per unit plus 0.2 per unit visitor spaces	177
Apartments 3-bed	29 No.	2.0 per unit plus 0.2 per unit visitor spaces	64
TOTAL	162 No.		276
		Bike parking standards	Parking required
Apartments	162 No.	1.0 per unit plus 0.2 per unit visitor spaces	195

Table 3-1: Parking required under Fingal County Council Development Plan Standards for residential component

It is proposed to provide 132 No. car parking spaces for the residential component, equating to 0.81 spaces per residential unit. This ratio is similar to the recently approved Techrete site 306102 (0.7 spaces per residential unit) located across the road from the proposed development. There will be a 10% provision of electrical vehicle charging points which equates to 13 EV parking spaces.

This level of provision is 48% of the quantum required under the Fingal County Development Plan standards. However, this provision must also be viewed in relation to the New Apartment Guidelines, the level of compliance with which is detailed within the section immediately below.

In terms of cycle parking provision, it is intended to provide 355 No. cycle parking spaces, 182% of the requirement under the Fingal Development Plan (195 No. spaces), but 87% of the quantum required under the National Cycle Manual (405 No.), which is widely regarded as constituting a very generous provision (1 No. per bed + 0.5 No. per unit).

3.2.3 CAR PARKING REQUIREMENTS FOR THE RESIDENTIAL COMPONENT BASED ON NEW APARTMENT GUIDELINES

Sustainable Urban Housing: Design Standards for New Apartments (Guidelines for Planning Authorities) was published by the Department of Housing, Planning and Local Government in March 2018.

Chapter 4 of this report refers specifically to revised car parking requirements for new apartment developments.

Its recommendations can be summarised as follows:

The quantum of car parking is dependent primarily on the location of the subject site. Three categories of location are defined:

Central and/or Accessible Urban Locations:

Apartments in central locations that are well served by public transport, in which situation car parking provision to be wholly eliminated or substantially reduced. These locations are most likely to be in cities, within 15 minutes walking distance of city centres or centrally located employment locations. These locations include sites within 10 minutes walking distance of DART, commuter rail or Luas stops or within 5 minutes walking distance of high frequency (min 10 minute peak hour frequency) bus services.

Intermediate Urban Locations

This applies to apartments in suburban/urban locations served by public transport or close to town centres or employment areas and particularly for housing schemes with more than 45 dwellings per hectare. For this category, planning authorities may consider a reduced overall car parking standard.

Peripheral and/or Less Accessible Urban Locations

An Bord Pleanála have recently confirmed in their decision to approve the development on the former Techrete site (306102), that the location of the subject site comes within the first category – a central/accessible urban location, within the Greater Dublin area, located within 200 metres of the Howth DART Station.

On the basis of this classification, it was concluded that a provision of between 0 and 1.0 parking spaces in total would be more than appropriate for the proposed development.

In relation to cycle parking provision, the New Apartment Guidelines refers to the National Cycle Manual, which requires 1 No. space per bedroom plus 0.5 spaces per unit for visitors. This equates to 406 No. spaces.

The proposed development provides 87% of the cycle parking required by the National Cycle Manual, which is seen as a very generous requirement. The proposed provision, 182% of the Fingal Development Plan requirement is seen as more than adequate to cater for the anticipated modal split.

3.3 COMMENT ON PARKING PROVISION

Given the centrality of the location for the proposed development under the guidance within the New Apartment Guidelines, the car parking provision is seen as totally consistent with its designation and is sustainable given the excellent public transport and soft mode links to the site.

4.0 TRIP GENERATION, DISTRIBUTION AND ASSIGNMENT FOR PROPOSED DEVELOPMENT

4.1 INTRODUCTION

The traffic impact of the proposed development is derived by assessing the trips generated by the proposal, taking the existing, day of opening and design year flows on the network, gauging the extent to which the superimposed flows from the proposed and adjacent committed developments will affect the efficiency of future network flows.

4.2 TRIPS GENERATED BY CANDIDATE SITE

The proposed development consists of 162 No. apartments

TRICS typically gives the following weekday morning and evening peak trip rates for apartments using Irish sites only where parking provision is not greater than 1.2 spaces per dwelling unit:

(These trip rates were used for the Claremont site at the former Techcrete factory:

		Weekday AM		Weekday PM	
		IN	OUT	IN	OUT
Apartments	Trips/Unit	0.04	0.19	0.16	0.06

Table 4-1: Peak hour trip rates for apartment development

The above TRICS trip rates give rise to the following weekday morning and evening peak trip rates for apartments:

		Weekday AM		Weekday PM	
		IN	OUT	IN	OUT
Units (No.)	162	7	31	26	10

Table 4-2: Peak hour flows generated by proposed apartments within development site

Thus, the proposal will result in a 2-way flow of 38 vehicles per hour in the morning peak, decreasing to 36 vehicles per hour in the evening peak (0.63 vehicles entering or exiting every minute during the morning peak, rising to 0.6 vehicles entering or exiting per minute during the evening peak).

4.3 TRIPS GENERATED BY NEARBY PLANNED DEVELOPMENTS (TECHCRETE PLUS BALSADDEN)

For the permitted Claremont development, former Techcrete site, the permitted development was predicted to generate 124 No. outbound trips and 64 No. inbound trips during the morning peak hour between 0800 and 0900, with 135 No. inbound trips and 101 No. outbound trips generated during the evening peak between 1700 and 1800.

Thus, the proposal will result in a 2-way flow of 188 vehicles per hour in the morning peak, increasing to 236 vehicles per hour in the evening peak (3 vehicles entering or exiting every minute during the morning peak, rising to 3.9 vehicles entering or exiting per minute during the evening peak

For the annulled Balscadden proposed development, this development was predicted to generate 41 No. outbound trips and 7 No. inbound trips during the morning peak hour between 0800 and 0900, with 27 No. inbound trips and 9 No. outbound trips generated during the evening peak between 1700 and 1800.

Thus, the proposal would have resulted in a 2-way flow of 48 vehicles per hour in the morning peak, decreasing to 36 vehicles per hour in the evening peak.

Combined, the permitted Claremont development and the now annulled Balscadden development, will generate a 2-way morning peak flow of 236 vehicles per hour and a 2-way evening peak flow of 272 vehicles per hour.

4.4 RELATIVE IMPACT OF PROPOSED DEVELOPMENT RELATIVE TO PROPOSED ADJACENT SCHEMES

The proposed development constitutes only 16% of the combined flows from the two adjacent developments at the Claremont site and now annulled Balscadden development during the morning peak hour, decreasing to 13% for the evening peak hour.

In relative terms, therefore, the impact of the proposed apartment development is low.

4.5 DISTRIBUTION OF GENERATED FLOWS FROM PROPOSED AND PLANNED DEVELOPMENTS

4.5.1 PROPOSED DEVELOPMENT

The same distribution assumptions are made as for the Claremont site assessment.

The incident flows along the R105 / Howth Road are relatively well balanced during both the morning and evening peaks.

In the morning peak in the interests of robustness of the analysis within this report, however, for exiting traffic (peak direction of flow), a 2:1 ratio will be assumed in favour of traffic exiting towards Sutton Cross. Of the one-third exiting towards Howth Village, 50% of trips are assumed to terminate in the local area, with the remaining 50% accessing Sutton Cross via Greenfield Road.

For traffic entering the development (non-peak direction of flow), 50% is assumed to enter from the Howth Village direction, with 50% from Sutton Cross via Howth Road.

At Sutton Cross, for exiting traffic (peak direction of flow) from Howth Road / Greenfield Road, 60% is assumed to exit to the Dublin Road, with 40% exiting to Station Road, while for traffic entering (non-peak direction of flow) from Sutton Cross, 50% will enter from Dublin Road and 50% from Station Road. 50% of this entering traffic will exit onto Howth Road, with 50% exiting onto Greenfield Road.

Diagram 3 within Appendix 3 details the assumed distributions for the AM peak hour generated flows for the proposed development.

In the evening peak, for exiting traffic (non-peak direction of flow), a 50:50 split will be assumed between traffic exiting towards Sutton Cross and Howth Village. Of the 50% exiting towards Howth Village, again 50% of trips are assumed to terminate in the local area, with the remaining 50% accessing Sutton Cross via Greenfield Road.

For traffic entering the development (peak direction of flow), one-third are assumed to enter from the Howth Village direction, with two-thirds from Sutton Cross via Howth Road.

At Sutton Cross, for exiting traffic (non-peak direction of flow) from Howth Road / Greenfield Road, 60% is assumed to exit to the Dublin Road, with 40% exiting to Station Road, while for traffic entering (peak direction of flow) from Sutton Cross, 60% will enter from Dublin Road and 40% from Station Road. 60% of this entering traffic will exit onto Howth Road, with 40% exiting onto Greenfield Road.

Diagram 4 within Appendix 3 details the assumed distributions for the PM peak hour generated flows for the proposed development.

4.5.2 PERMITTED DEVELOPMENT CLAREMONT (FORMER TECHCRETE SITE) & BALSCADDEN DEVELOPMENT (NOW ANNULLED)

The same assumptions as above will be made for the flows generated by the permitted Claremont development and the now annulled Balscadden development.

These assumed distributions are detailed for the morning and evening peak hours within Diagrams 5 and 6 respectively within Appendix 3.

4.6 TRIP ASSIGNMENT

The 2014 Traffic and Transport Assessment Guidelines published by the NRA requires that the relevant junctions be analysed for the existing situation, the year of opening (2023) with the proposed and adjacent developments in place, the design year 1 (year of opening plus 5) with the proposed and adjacent developments in place, and the design year 2 (year of opening plus 15) with the proposed and adjacent developments in place. In order to bring focus to the analysis, design year 1 has been omitted from those junctions fully analysed.

An annual growth rate of 1.4% has been assumed for the period 2019 to 2030, decreasing to 0.5% for 2031 to 2037, based on the medium growth estimate for Fingal County Council published by TII in 2017 (PE-PAG-02017).

The 2023 Do-Nothing ('without development') scenario is derived by factoring the survey results in Diagrams 1 and 2 up by 4.2% $((1.014)^3 - 1 = 0.042)$. The 2023 Do-Nothing ('with adjacent development') scenario is derived by adding the development flows detailed within Diagrams 3 and 4 to these factored network flows. The 2023 Do-Something ('with proposed and adjacent development') scenario is derived by adding the development flows detailed within Diagrams 3, 4, 5 and 6 to these factored network flows.

The 2038 Do-Nothing ('without development') scenario is derived by factoring the survey results in Diagrams 1 and 2 up by 20.7% $((1.014)^{11} - 1) / ((1.005)^7 - 1) = 0.207$. The 2038 Do-Nothing ('with adjacent development') scenario is derived by adding the development flows detailed within Diagrams 3 and 4 to these factored network flows. The 2038 Do-Something ('with proposed and adjacent development') scenario is derived by adding the development flows detailed within Diagrams 3, 4, 5 and 6 to these factored network flows.

The 2038 analysis constitutes a significantly conservative analysis, as given current transport policy in the Greater Dublin Area, where use of the private car for the trip to work is being actively discouraged, and use of public transport and soft modes actively encouraged, it is highly unlikely that an increase in traffic volumes of 21% from now until 2038 will take place.

In reality, it could reasonably be assumed going forward that traffic volume increases during the morning and evening peaks will be marginal over the coming years. The working from home trend, post covid, may reduce the volume of this increase however.

4.7 IMPACT OF GENERATED FLOWS ON CRITICAL JUNCTIONS

Table 4-3 details the increases in traffic at the 4 No. existing junctions, plus the proposed development entrance:

		Network Flows		Committed Flows (as % of network)		Generated Flows (as % of network)	
		AM	PM	AM	PM	AM	PM
Sutton Crossroads	site 1	1944	1602	184 (9.4%)	227 (14.2%)	32 (1.6%)	30 (1.9%)
Church Road / Howth Road	site 2	1055	972	146 (13.8%)	164 (16.8%)	25 (2.4%)	22 (2.3%)
Offington Park / Howth Road	site 3	945	917	146 (15.4%)	164 (17.9%)	25 (2.6%)	22 (2.4%)
Harbour Road / Church Street	site 4	730	719	96 (13.2%)	126 (17.5%)	14 (1.9%)	16 (2.2%)
Development entrance		730	719	146 (20%)	164 (23%)	38 (5.2%)	36 (5%)

Table 4-3: October 2019 surveys at 4 No. critical junctions, flows from committed developments incident each junction and flows from subject site incident on each junction.

The 2014 Traffic and Transport Assessment Guidelines requires the impact of the additional traffic volumes on the critical nearby junctions to be assessed in detail if:

- Development flows exceed 10% of existing turning movements at the two relevant junctions;
- Development flows exceed 5% of turning movements if the location has the potential to become congested.

It can be seen that the adjacent developments are in excess of the threshold of 5% and 10% thresholds at all five junctions. However, the proposed development is significantly below any thresholds where a TTA would be required.

In the interests of robustness, however, the overall impact of the permitted Claremont scheme, the annulled Balcadden scheme and the proposed development will be analysed within this analysis.

(It should be noted that two other junctions were initially analysed within the Claremont TTA and were not analysed in any further detail as the generated flows were significantly below the stated thresholds for requiring further analysis.)

5.0 TRAFFIC IMPACT ASSESSMENT OF 5 NO. CRITICAL JUNCTIONS IN PROXIMITY TO THE SUBJECT SITE

5.1 INTRODUCTION

The traffic analysis will analyse the performance of the following 1 No. signalised and 4 No. priority intersections for the following scenarios:

Sutton Cross

- Existing flows (AM and PM peak)
- 2023 flows without development in place (AM and PM peak)
- 2023 flows with adjacent development but without proposed development in place (AM and PM peak)
- 2023 flows with proposed and adjacent development in place (AM and PM peak)
- 2038 flows without development in place (AM and PM peak)
- 2038 flows with adjacent development but without proposed development in place (AM and PM peak)
- 2038 flows with proposed and adjacent development in place (AM and PM peak)

Development Entrance

- 2023 flows with proposed and adjacent development in place (AM and PM peak)
- 2038 flows with proposed and adjacent development in place (AM and PM peak)

Offington Park / Howth Road

- 2023 flows with and without proposed and adjacent development in place (AM and PM peak)
- 2038 flows with proposed and adjacent development in place (AM and PM peak)

Church Road / Howth Road

- 2023 flows with and without proposed and adjacent development in place (AM and PM peak)
- 2038 flows with proposed and adjacent development in place (AM and PM peak)

Harbour Road / Church Street

- 2023 flows with and without proposed and adjacent development in place (AM and PM peak)
- 2038 flows with proposed and adjacent development in place (AM and PM peak)

The OSCADY programme from the Junctions 9 suite will be used to analysis the signalised junctions and the PICADY programme from the Junctions 9 suite will be utilised to analyse the priority for all relevant scenarios.

5.2 ANALYSIS JUNCTION OF SUTTON CROSSROADS SIGNALISED JUNCTION

5.2.1 GEOMETRIC PARAMETERS

For the junction in question, the following geometric characteristics have been used for the purposes of this assessment:

Howth Road approach (Arm A)

2 No. lanes, inside lane for straight-ahead and left-turning traffic (4 metres wide), outside lane for right-turning only (3 metres wide).

Greenfield Road South (Arm B)

1 No. lanes for all directions of traffic (3 metres wide).

Dublin Road (Arm C)

2 No. lanes, inside lane for left-turning and straight-ahead traffic (3 metres wide), outside lane for right-turning traffic only (3 metres wide).

Station Road North (Arm D)

2 No. lanes, inside lane for left-turning traffic only (3 metres wide), outside lane for straight-ahead and right-turning only (3 metres wide).

5.2.2 ANALYSIS OF AM AND PM PEAK HOUR FLOWS FOR THE 7 NO. SCENARIOS

Table 5-1 immediately below summarises the critical flows, capacities, RFC's and queue lengths for the morning and evening peaks for each of the 2 No. scenarios:

Scenario No.1	EXISTING AM PEAK FLOWS				EXISTING PM PEAK FLOWS			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Howth Rd (Arm A)	63	79.58	0.79	25	64	104.23	0.61	17
Carrickrock Rd (Arm B)	118	167.41	0.71	14	119	147.25	0.81	17
Dublin Rd (Arm C)	90	95.02	0.95	24	50	87.42	0.57	16
Station Road (Arm D)	83	184.21	0.45	15	67	229.76	0.29	10
Scenario No.2	2023 AM PEAK FLOWS (Do-Nothing)				2023 PM PEAK FLOWS (Do-Nothing)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Howth Rd (Arm A)	66	79.58	0.83	17	67	104.23	0.64	18
Carrickrock Rd (Arm B)	122	167.44	0.73	15	124	147.29	0.84	18
Dublin Rd (Arm C)	94	95.02	0.99	27	52	87.42	0.59	16
Station Road (Arm D)	86	184.44	0.47	15	70	229.76	0.30	10
Scenario No.3	2023 AM PEAK FLOWS (Do-Adjacent)				2023 PM PEAK FLOWS (Do-Adjacent)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Howth Rd (Arm A)	129	149.03	0.87	31	72	101.99	0.71	20
Carrickrock Rd (Arm B)	129	161.87	0.80	17	132	147.06	0.90	20
Dublin Rd (Arm C)	96	92.49	1.04	30	94	141.33	0.67	19
Station Road (Arm D)	89	179.01	0.50	16	80	227.52	0.35	11
Scenario No.4	2023 AM PEAK FLOWS (Do-Proposed + Adjacent)				2023 AM PEAK FLOWS (Do-Proposed + Adjacent)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Howth Rd (Arm A)	132	149.06	0.89	32	73	99.75	0.73	20
Carrickrock Rd (Arm B)	128	160.21	0.80	17	133	144.18	0.92	21
Dublin Rd (Arm C)	97	92.49	1.05	31	96	147.16	0.65	19
Station Road (Arm D)	89	179.01	0.50	16	82	223.04	0.37	12
Scenario No.5	2038 AM PEAK FLOWS (Do-Nothing)				2038 PM PEAK FLOWS (Do-Nothing)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Howth Rd (Arm A)	76	79.58	0.96	36	77	101.99	0.75	21
Carrickrock Rd (Arm B)	141	164.72	0.86	19	144	150.08	0.96	24
Dublin Rd (Arm C)	109	97.56	1.12	37	60	87.42	0.69	19
Station Road (Arm D)	100	181.43	0.55	18	81	229.76	0.35	12
Scenario No.6	2038 AM PEAK FLOWS (Do-Adjacent)				2038 PM PEAK FLOWS (Do-Adjacent)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Howth Rd (Arm A)	147	143.08	1.03	48	83	99.75	0.83	24
Carrickrock Rd (Arm B)	145	160.29	0.91	22	152	149.88	1.01	28
Dublin Rd (Arm C)	111	97.56	1.14	39	106	141.33	0.75	22
Station Road (Arm D)	103	178.79	0.58	19	91	227.52	0.40	13
Scenario No.7	2038 AM PEAK FLOWS (Do-Proposed + Adjacent)				2038 AM PEAK FLOWS (Do-Proposed + Adjacent)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Howth Rd (Arm A)	151	143.11	1.06	53	83	99.75	0.83	24
Carrickrock Rd (Arm B)	147	163.06	0.91	21	152	147.08	1.03	30
Dublin Rd (Arm C)	111	95.02	1.17	41	108	144.25	0.75	23
Station Road (Arm D)	103	181.61	0.57	19	93	225.28	0.41	13

Table 5-1: Critical flows, capacities, ratios of flow to capacity and queue lengths for each 15-minute interval during the morning and evening peak hours for each scenario

The above analysis indicates that the junction is at present heavily loaded and operating at capacity on major approaches, with between 20 and 25 vehicles queuing on major approaches during both peak hours. Queuing and delays are thus significant during both peak hours (Scenario No. 1).

By 2023, assuming network flow increases of 4.2%, until the projected day of opening of the proposed development, without any development in place, maximum queuing will increase by up to 3 No. vehicles during both peaks relative to the existing situation (Scenario No. 2).

By 2023, assuming network flow increases of 4.2%, until the projected day of opening of the proposed development, with adjacent planned development in place but without the proposed development in place, maximum queuing will increase to a maximum of 31 No. vehicles over both peaks, with one arm at 104% capacity (Scenario No. 3).

By 2023, assuming network flow increases of 4.2%, until the projected day of opening of the proposed development, with both adjacent planned development and the proposed development in place, maximum queuing will increase marginally on Scenario No. 3 to a maximum of 32 No. vehicles over both peaks, with one arm at 105% capacity, up only 1% on Scenario 3 (Scenario No. 4).

By 2038, assuming network flow increases of 21%, 15 years after the projected day of opening of the proposed development, without any development in place, maximum queuing will increase to a maximum of 37 No. vehicles over both peaks, with one arm at 112% capacity (Scenario No. 5).

By 2038, assuming network flow increases of 21%, 15 years after the projected day of opening of the proposed development, with adjacent planned development in place but without the proposed development in place, maximum queuing will increase to a maximum of 48 No. vehicles over both peaks, with one arm at 114% capacity and one at 103% capacity (Scenario No. 6).

By 2038, assuming network flow increases of 21%, 15 years after the projected day of opening of the proposed development, with both adjacent planned development and the proposed development in place, maximum queuing will increase marginally by 9.4% to a maximum of 53 No. vehicles over both peaks, with one arm at 117% capacity and one at 106% capacity, up only 3% on Scenario 6 (Scenario No. 7).

Thus, in overall terms, Sutton Crossroads at present operates at capacity with significant queuing and delays. The permitted adjacent development at Claremont and the now annulled Balscadden development, will significantly increase queuing and delays at this location, with the proposed development only adding marginally to these impacts.

We note the comments of An Bord Pleanála's Inspector in the assessment of the Claremont development; The traffic congestion that occurs in the town centre and at Sutton Cross is not a result of a growing population in Howth because no such growth has occurred. Rather it is a result of a more dispersed pattern of settlement in and around Howth and the city that depends more on the use private car to access services, employment and, especially in the case of Howth, places of recreation. Refusing permission for the proposed development or significantly reducing its scale would do nothing to alleviate traffic congestion in Howth or at Sutton Cross.

5.3 ANALYSIS OF DEVELOPMENT ENTRANCE PRIORITY JUNCTION

5.3.1 GEOMETRIC PARAMETERS

For the junction in question, the analysis assumes that the Howth Road major carriageway is 7.3 metres wide in the vicinity of the entrance (2.2 metre wide right-turning lane assumed for westbound traffic), with the Development Entrance minor approach assumed to consist of 1 No. 3.0 metre wide lane.

5.3.2 ANALYSIS OF AM AND PM PEAK HOUR FLOWS FOR THE 2 NO. SCENARIOS

Table 5-2 immediately below summarises the critical flows, capacities, RFC's and queue lengths for the morning and evening peaks for each of the 2 No. scenarios:

Scenario No.1	2023 AM PEAK FLOWS (Do-Proposed + Adjacent)				2023 AM PEAK FLOWS (Do-Proposed + Adjacent)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Development exit onto Howth Road (B-AC)	8	98.62	0.08	0.1	2	99.35	0.02	0.0
Right turn into site from Howth Road (C-B)	1	115.47	0.01	0.0	4	121.64	0.03	0.0
Scenario No.2	2038 AM PEAK FLOWS (Do-Proposed + Adjacent)				2038 AM PEAK FLOWS (Do-Proposed + Adjacent)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Development exit onto Howth Road (B-AC)	8	91.23	0.09	0.1	2	92.78	0.02	0.0
Right turn into site from Howth Road (C-B)	1	110.84	0.01	0.0	4	117.45	0.03	0.0

Table 5-2: Critical flows, capacities, ratios of flow to capacity and queue lengths for each 15-minute interval during the morning and evening peak hours for each scenario

All approaches will be within capacity at all times during both peaks on both the day of opening and fifteen years thereafter in 2038.

A minimum of 91% spare capacity exists on all opposed movement over all peak times.

Queuing at present is at very low levels for turning movements at the junctions during the morning and evening peaks, with queuing on any opposed movement never exceeding 1 vehicle.

5.4 ANALYSIS OF HOWTH ROAD / CHURCH ROAD PRIORITY JUNCTION

5.4.1 GEOMETRIC PARAMETERS

For the junction in question, the analysis assumes that the Howth Road major carriageway is 7.3 metres wide in the vicinity of the entrance (2.2 metre wide right-turning lane assumed for westbound traffic), with the Church Road minor approach assumed to consist of 1 No. 3.0 metre wide lane.

5.4.2 ANALYSIS OF AM AND PM PEAK HOUR FLOWS FOR THE 2 NO. SCENARIOS

Table 5-3 immediately below summarises the critical flows, capacities, RFC's and queue lengths for the morning and evening peaks for each of the 2 No. scenarios:

Scenario No.1	2023 AM PEAK FLOWS (Do-Proposed + Adjacent)				2023 AM PEAK FLOWS (Do-Proposed + Adjacent)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Church Road exit onto Howth Road (B-AC)	63	116.19	0.54	1.1	17	124.52	0.14	0.2
Right turn into Church Rd from Howth Rd (C-B)	35	122.52	0.29	0.4	26	126.05	0.21	0.3
Scenario No.2	2038 AM PEAK FLOWS (Do-Proposed + Adjacent)				2038 AM PEAK FLOWS (Do-Proposed + Adjacent)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Church Road exit onto Howth Road (B-AC)	74	109.43	0.68	1.9	19	120.57	0.16	0.2
Right turn into Church Rd from Howth Rd (C-B)	41	119.21	0.34	0.5	30	122.74	0.24	0.3

Table 5-3: Critical flows, capacities, ratios of flow to capacity and queue lengths for each 15-minute interval during the morning and evening peak hours for each scenario

All approaches will be within capacity at all times during both peaks on both the day of opening and fifteen years thereafter in 2038.

A minimum of 32% spare capacity exists on all opposed movement over all peak times.

Queuing at present is at very low levels for turning movements at the junctions during the morning and evening peaks, with queuing on any opposed movement never exceeding 2 vehicles.

5.5 ANALYSIS OF HOWTH ROAD / OFFINGTON PARK PRIORITY JUNCTION

5.5.1 GEOMETRIC PARAMETERS

For the junction in question, the analysis assumes that the Howth Road major carriageway is 7.3 metres wide in the vicinity of the entrance (2.2 metre wide right-turning lane assumed for westbound traffic), with the Offington Park minor approach assumed to consist of 1 No. 3.0 metre wide lane.

5.5.2 ANALYSIS OF AM AND PM PEAK HOUR FLOWS FOR THE 2 NO. SCENARIOS

Table 5-4 immediately below summarises the critical flows, capacities, RFC's and queue lengths for the morning and evening peaks for each of the 2 No. scenarios:

Scenario No.1	2023 AM PEAK FLOWS (Do-Proposed + Adjacent)				2023 AM PEAK FLOWS (Do-Proposed + Adjacent)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Offington Pk exit onto Howth Road (B-AC)	54	89.78	0.60	1.4	30	90.56	0.33	0.5
Right turn into Offington Pk from Howth Rd (C-B)	26	114.59	0.23	0.3	20	121.64	0.16	0.2
Scenario No.2	2038 AM PEAK FLOWS (Do-Proposed + Adjacent)				2038 AM PEAK FLOWS (Do-Proposed + Adjacent)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Offington Pk exit onto Howth Road (B-AC)	63	82.06	0.77	2.8	35	84.60	0.41	0.7
Right turn into Church Rd from Howth Rd (C-B)	30	110.18	0.27	0.4	23	117.67	0.20	0.2

Table 5-4: Critical flows, capacities, ratios of flow to capacity and queue lengths for each 15-minute interval during the morning and evening peak hours for each scenario

All approaches will be within capacity at all times during both peaks on both the day of opening and fifteen years thereafter in 2038.

A minimum of 23% spare capacity exists on all opposed movement over all peak times.

Queuing at present is at very low levels for turning movements at the junctions during the morning and evening peaks, with queuing on any opposed movement never exceeding 3 vehicles.

5.6 ANALYSIS OF HARBOUR ROAD / CHURCH STREET PRIORITY JUNCTION

5.6.1 GEOMETRIC PARAMETERS

For the junction in question, the analysis assumes that the Harbour Road major carriageway is 7.3 metres wide in the vicinity of the entrance (2.2 metre wide right-turning lane assumed for westbound traffic), with the Church Street minor approach assumed to consist of 1 No. 3.0 metre wide lane.

5.6.2 ANALYSIS OF AM AND PM PEAK HOUR FLOWS FOR THE 2 NO. SCENARIOS

Table 5-5 immediately below summarises the critical flows, capacities, RFC's and queue lengths for the morning and evening peaks for each of the 2 No. scenarios:

Scenario No.1	2023 AM PEAK FLOWS (Do-Proposed + Adjacent)				2023 AM PEAK FLOWS (Do-Proposed + Adjacent)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Church St exit onto Harbour Rd (B-AC)	41	134.88	0.30	0.4	28	139.29	0.20	0.2
Right turn into Church St from Harbour Rd (C-B)	55	124.06	0.44	0.8	42	133.98	0.31	0.4
Scenario No.2	2038 AM PEAK FLOWS (Do-Proposed + Adjacent)				2038 AM PEAK FLOWS (Do-Proposed + Adjacent)			
	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)	Flow (veh/TS)	Cap. (veh/TS)	RFC (-)	Avg. queue (vehicles)
Church St exit onto Harbour Rd (B-AC)	47	131.78	0.36	0.5	32	136.77	0.23	0.3
Right turn into Church St from Harbour Rd (C-B)	63	120.98	0.52	1.1	48	132.00	0.36	0.6

Table 5-5: Critical flows, capacities, ratios of flow to capacity and queue lengths for each 15-minute interval during the morning and evening peak hours for each scenario

All approaches will be within capacity at all times during both peaks on both the day of opening and fifteen years thereafter in 2038.

A minimum of 48% spare capacity exists on all opposed movement over all peak times.

Queuing at present is at very low levels for turning movements at the junctions during the morning and evening peaks, with queuing on any opposed movement never exceeding 2 vehicles.

6.0 SUMMARY COMMENTS ON TRAFFIC IMPACT OF PROPOSED DEVELOPMENT

6.1 SUMMARY OF ANALYSIS

This document contains a Traffic and Transport Assessment for a proposed development located at Deer Park, Howth, Co. Dublin, comprising 162 No. apartments and providing 132 No. car parking spaces.

The function of this TTA is to quantify the existing transport environment in terms of the vehicular flows incident on it and to identify and assess the level of transport impact generated by the vehicular trips generated by both the proposed residential development and adjacent permitted developments as required by FCC.

This TTA has carried out a range of assessments for the existing situation, within the year of opening in 2023, and within 2038 design year (year of opening plus 15).

6.2 CONCLUSIONS FROM ANALYSIS

Based on the data and evaluations within this TTA, the following conclusions can be made:

1. The network analysis within the TTA indicates that 3 of the 4 No. existing critical junctions in the vicinity of the proposed development plus the proposed development entrance presently work well within capacity and will continue to do so in 2023 the projected year of opening and in 2038, 15 years thereafter.
2. Sutton crossroads is at present at capacity and other adjacent permitted developments will result in it being over capacity in future years. The proposed development will add relatively little to further congestion at this location. The volumes generated by the proposed development constitute a very low proportion of these committed flows from adjacent developments.
3. The subject site is highly accessible to pedestrians and cyclists in the vicinity of Howth Road and its environs, with excellent pedestrian connectivity from the proposed development eastwards towards Howth Village and access to the DART.

4. Future proposals as stated within the GDA Cycle Network Plan include a secondary cycle route is planned along Howth Road and Carrickbrack Road, which will connect the subject site to all parts of Howth, southwards towards the city centre and north-westwards towards Portmarnock, Malahide and Swords. In addition, the proposed East Coast Greenway will run on the northern edge of the site, connecting Howth to the greenway network in the Greater Dublin area.
5. The site is well served by public transport, with the bus route 31/a along Howth Road towards the city centre, and the 31b Route along Carrickbrack Road towards the city centre, and the DART within 400 metres of the proposed development.
6. This document is a robust assessment that includes the permitted Claremont development to the north of Howth Road and the formerly permitted Balscadden SHD development. The latter is included as it is reasonably anticipated having regard to the sites zoning and planning history that a similar scheme will be progressed on the site in the future.

6.3 MITIGATION

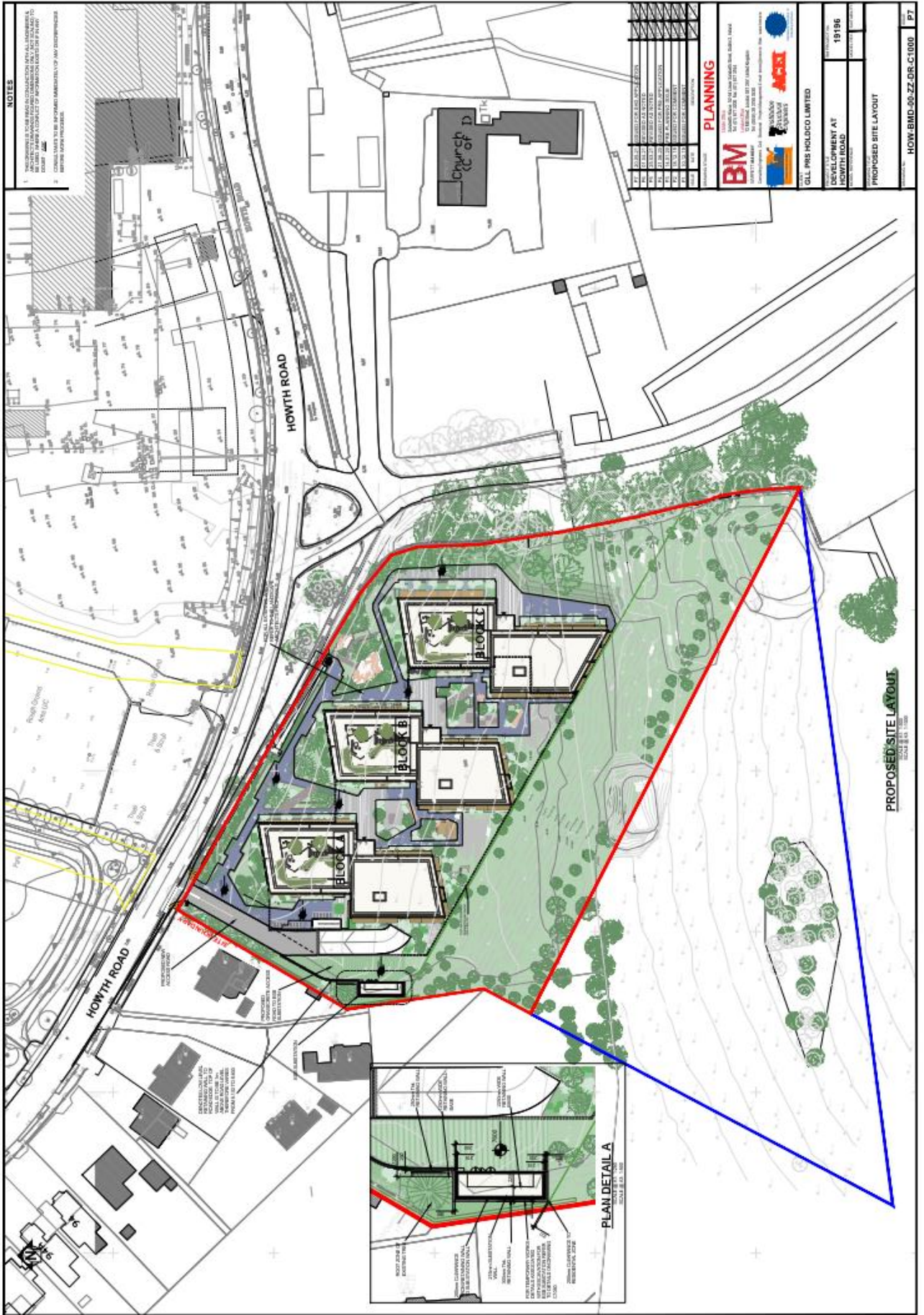
In the context of the overall volume of flows generated by all proposed development in the area, the importance of implementing a coherent parking and mobility policy for the area becomes of significant importance, as such policies will minimise the impact of private car traffic and will also be in keeping with the sustainable transport policy presently advocated for the Greater Dublin Area – please also refer to BMCE Mobility Management Plan (MMP) document 19.196-IR-03 which accompanies this SHD application.

The MMP is compiled with the aim of guiding the delivery and management of coordinated initiatives by the applicant. The MMP ultimately seeks to encourage sustainable travel practices for all journeys to and from the proposed development.

APPENDIX

1

SITE LAYOUT
PLAN



NOTES

1. THE DEVELOPER SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AGENCIES AND AGENCIES. ALL WORK SHALL BE IN ACCORDANCE WITH ALL APPLICABLE CODES AND REGULATIONS.
2. THE DEVELOPER SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AGENCIES AND AGENCIES. ALL WORK SHALL BE IN ACCORDANCE WITH ALL APPLICABLE CODES AND REGULATIONS.
3. THE DEVELOPER SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AGENCIES AND AGENCIES. ALL WORK SHALL BE IN ACCORDANCE WITH ALL APPLICABLE CODES AND REGULATIONS.

PLANNING		B.M.	ACCI	ILLINOIS POLICE LIMITED	DEVELOPMENT AT HOWTH ROAD	19196	PROPOSED SITE LAYOUT	HOW-BMD-09-ZZ-DR-C-1000	PT
NO. 19196-01	NO. 19196-02								

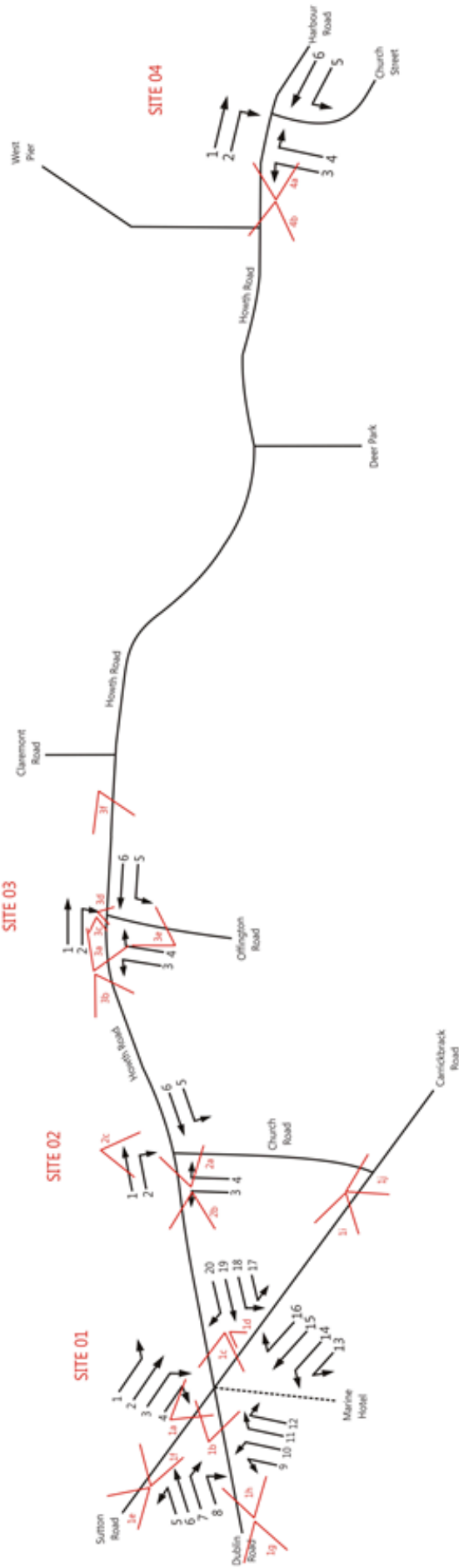


APPENDIX

2

TRAFFIC
SURVEYS
OCTOBER 2019

JUNCTION TURNING MOVEMENT COUNTS



DATE: 22nd October 2019		SITE: 01		DATE: 22nd October 2019		SITE: 01		DATE: 22nd October 2019		SITE: 01		DATE: 22nd October 2019		SITE: 01									
DAY: Tuesday		LOCATION: Sutton Cross		DAY: Tuesday		LOCATION: Sutton Cross		DAY: Tuesday		LOCATION: Sutton Cross		DAY: Tuesday		LOCATION: Sutton Cross									
MOVEMENT 8		MOVEMENT 9		MOVEMENT 10		MOVEMENT 11		MOVEMENT 12		MOVEMENT 13		MOVEMENT 14											
PCU	CAR	LEV	OSVI	OSV2	BUS	TOT	PCU	CAR	LEV	OSVI	OSV2	BUS	TOT	PCU	CAR	LEV	OSVI	OSV2	BUS	TOT	PCU		
7	0	0	0	0	1	1	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15	2	0	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
24	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
37	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
83	2	0	0	0	1	3	4	H/TOT	2	0	0	0	0	2	2	1	0	0	0	0	3	3	1
48	0	0	0	0	0	0	0	08:00	2	0	0	0	0	2	2	0	1	0	0	0	1	1	1
89	0	0	1	0	0	1	2	08:15	0	0	0	0	0	0	0	0	0	0	0	0	1	2	4
56	2	0	0	0	0	2	2	08:30	2	0	0	0	0	2	2	1	0	0	0	0	1	1	2
52	1	0	0	0	0	1	1	08:45	2	0	0	0	0	2	2	1	0	0	0	0	1	1	0
244	3	0	1	0	0	4	5	H/TOT	6	0	0	0	0	6	6	2	1	0	0	0	3	3	1
25	3	0	0	0	0	3	3	09:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
25	1	0	0	0	0	1	1	09:15	2	0	0	0	0	2	2	1	0	0	0	0	1	1	1
25	1	0	0	0	0	1	1	09:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
28	3	0	0	0	0	3	3	09:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
82	8	0	0	0	0	8	8	H/TOT	2	0	0	0	0	2	2	2	1	0	0	0	1	1	1
20	4	0	0	0	0	4	4	10:00	3	0	0	0	0	3	3	0	0	0	0	0	0	0	0
24	0	0	0	0	0	0	0	10:15	3	0	0	0	0	3	3	0	0	0	0	0	0	0	0
30	4	0	0	0	0	4	4	10:30	1	0	0	0	0	1	1	2	0	0	0	0	2	2	0
24	1	0	0	0	0	1	1	10:45	4	0	0	0	0	4	4	0	1	0	0	0	1	1	0
97	9	0	0	0	0	9	9	H/TOT	11	0	0	0	0	11	11	2	1	0	0	0	3	3	0
17	2	1	0	0	0	3	3	11:00	4	0	0	0	0	4	4	1	0	0	0	0	1	1	0
30	2	0	0	0	0	2	2	11:15	1	0	0	0	0	1	1	1	0	0	0	0	1	2	3
31	3	1	0	0	0	4	4	11:30	4	1	0	0	0	5	5	1	0	0	0	0	1	1	0
34	5	2	0	0	0	7	7	11:45	9	0	0	0	0	9	9	1	0	0	0	0	1	1	0
112	12	4	0	0	0	16	16	H/TOT	18	1	0	0	0	19	19	4	0	0	0	0	4	4	2
25	1	0	0	0	0	1	1	12:00	2	1	0	0	0	3	3	0	1	0	0	0	1	1	0
38	2	0	0	0	0	2	2	12:15	7	1	0	0	0	8	8	1	0	0	0	0	1	1	0
25	4	0	0	0	0	4	4	12:30	6	1	0	0	0	7	7	1	0	0	0	0	1	1	0
37	7	0	0	0	0	7	7	12:45	5	0	0	0	0	5	5	1	0	0	0	0	1	1	0
524	14	0	0	0	0	14	14	H/TOT	20	3	0	0	0	23	23	3	1	0	0	0	4	4	1

1	DATE: 22nd October 2019				SITE: 01				LOCATION: Sutton Cross				DATE: 22nd October 2019				PCU's Through Junction											
	DAY: Tuesday				DAY: Tuesday				DAY: Tuesday				DAY: Tuesday															
MOVEMENT 15		MOVEMENT 16		MOVEMENT 17		MOVEMENT 18		MOVEMENT 19		MOVEMENT 20		MOVEMENT 20		MOVEMENT 20														
PCU	CAR	LEV	OSV1	OSV2	BUS	TOT	PCU	CAR	LEV	OSV1	OSV2	BUS	TOT	PCU	CAR	LEV	OSV1	OSV2	BUS	TOT	PCU							
26	13	1	0	1	0	15	16	0	0	0	0	0	0	44	2	0	0	0	1	47	48	21	1	0	0	22	22	186
38	13	1	0	0	0	14	14	0	0	0	0	0	0	45	7	0	1	0	53	54	29	1	0	0	30	30	245	
44	33	0	0	0	0	33	33	0	0	0	0	0	0	38	1	0	0	1	40	41	29	2	1	1	0	33	35	292
41	34	2	1	2	0	39	42	1	0	0	0	1	1	34	2	1	0	0	37	38	28	1	2	0	0	31	32	340
151	93	4	1	3	0	101	105	1	0	0	0	1	1	161	12	1	1	2	177	181	107	5	3	1	0	119	119	1063
49	51	1	3	1	0	56	59	0	0	0	0	0	0	47	4	1	0	1	53	55	32	2	2	0	0	36	37	424
69	44	0	0	0	0	44	44	2	0	0	0	2	2	63	5	0	0	0	68	68	40	3	0	1	0	44	45	509
63	36	1	0	0	2	39	41	2	0	0	0	2	2	95	6	1	0	1	103	105	57	3	2	0	0	62	63	598
63	48	4	1	0	0	53	54	1	0	0	0	1	1	85	2	3	0	0	90	92	42	6	0	1	0	49	50	461
244	170	6	4	1	2	192	197	5	0	0	0	5	7	299	17	5	0	2	314	319	171	14	4	2	0	191	196	1993
66	51	3	1	1	0	56	58	2	0	0	0	2	2	74	5	1	1	1	82	85	62	9	2	0	0	73	74	448
39	38	2	1	0	0	41	42	1	0	0	0	1	1	81	5	0	0	0	86	86	49	3	0	0	0	52	52	406
27	19	1	0	0	1	21	22	1	0	0	0	1	1	55	6	1	0	1	63	65	44	9	1	0	0	54	55	305
28	22	0	1	2	0	25	28	2	0	0	0	2	2	58	7	1	0	2	66	71	19	3	0	0	1	23	24	348
162	130	6	3	3	1	143	149	6	0	0	0	6	6	288	23	3	1	4	299	306	174	24	3	0	1	202	205	1507
40	21	5	2	0	0	28	29	5	0	0	0	5	5	46	7	0	0	2	55	57	40	9	2	0	0	51	52	371
30	24	2	2	0	0	28	29	1	0	0	0	1	1	60	6	2	0	1	69	71	35	5	2	0	0	42	43	334
36	27	3	1	1	0	32	34	3	0	0	0	3	3	56	5	2	0	1	64	66	34	5	6	0	0	45	48	368
47	34	0	0	0	0	34	34	1	0	0	0	1	1	67	4	2	2	2	77	83	47	4	4	1	0	56	59	428
162	106	5	1	0	0	122	126	10	0	0	0	10	10	229	22	6	2	6	265	277	156	23	14	1	0	194	202	1501
43	25	3	0	0	0	28	28	4	0	0	0	4	4	47	4	0	0	0	51	51	50	8	2	0	1	61	63	358
50	22	1	0	0	0	23	23	1	0	0	0	1	1	58	6	2	0	1	67	68	42	3	0	0	0	45	45	379
22	15	4	2	1	0	22	24	1	0	0	0	1	1	62	7	0	0	1	70	71	38	11	3	1	0	53	56	355
39	30	4	2	1	0	37	39	1	0	0	0	1	1	62	9	0	0	0	71	71	43	11	4	1	0	59	62	430
154	92	12	4	2	0	110	115	7	0	0	0	7	7	229	26	2	0	2	259	262	173	33	9	2	1	218	226	1522
34	19	4	0	0	0	23	23	0	0	0	0	0	0	64	2	2	0	1	68	71	27	9	2	1	0	39	41	347
30	21	1	1	0	0	23	24	0	0	0	0	0	0	53	4	3	1	1	62	66	47	10	0	0	0	57	57	328
34	19	2	0	0	0	21	21	4	0	0	0	4	4	66	6	1	0	1	74	76	55	7	1	0	0	63	64	419
36	19	3	2	0	0	24	25	2	2	0	0	4	4	78	7	1	0	1	87	89	38	1	1	0	0	40	41	442
134	78	10	3	0	0	91	93	6	2	0	0	8	8	281	19	7	1	4	292	301	167	27	4	1	0	189	202	1535

1	DATE: 22nd October 2019														DATE: 22nd October 2019	PCU's Through Junction																
	SITE: 01																DATE: 22nd October 2019															
	LOCATION: Sutton Cross																	DATE: 22nd October 2019														
	DAY: Tuesday																		DATE: 22nd October 2019													
DAY: Tuesday																																
MOVEMENT 15			MOVEMENT 16			MOVEMENT 17			MOVEMENT 18			MOVEMENT 19			MOVEMENT 20					PCU												
PCU	CAR	LGV	OGV1/2	BUS	TOT	PCU	CAR	LGV	OGV1/2	BUS	TOT	PCU	CAR	LGV	OGV1/2	BUS	TOT	PCU														
					TIME																											
39	25	2	0	0	0	2	2	1	0	0	3	3	2	0	0	2	2	53	3	1	0	1	58	60	49	9	1	1	0	60	62	
23	19	2	1	0	0	2	2	0	0	0	2	2	4	0	0	4	4	68	5	0	1	1	75	77	47	3	4	0	1	55	58	
63	26	2	1	0	0	0	0	1	0	0	2	2	3	0	0	3	3	57	4	3	0	1	65	68	36	7	2	0	0	45	46	
43	30	7	3	1	0	41	44	1	0	0	0	1	1	0	0	0	0	52	2	2	0	0	56	57	40	3	2	0	0	45	46	
167	100	13	5	1	0	119	123	5	0	0	5	5	9	0	0	0	9	230	14	6	1	3	254	261	172	22	9	1	1	205	212	
37	27	4	2	0	0	33	34	3	0	0	3	4	4	1	0	0	2	2	59	6	0	0	1	66	67	46	9	0	0	55	55	
38	31	4	0	0	0	35	35	1	0	0	0	1	3	1	0	0	1	1	57	6	0	0	1	64	65	42	5	1	0	0	48	49
42	33	4	0	0	0	37	37	0	0	0	0	3	3	0	0	0	0	68	3	1	0	1	73	75	51	8	1	0	0	60	61	
55	28	4	0	1	0	33	34	1	0	0	0	1	2	0	0	0	1	2	77	2	0	0	0	79	79	48	4	1	0	0	53	54
171	119	18	2	1	0	158	160	5	0	0	5	5	8	5	0	0	0	191	17	1	0	3	202	206	187	25	3	0	0	166	178	
43	24	3	2	0	0	29	30	1	0	0	0	1	2	0	0	0	0	62	6	0	0	1	69	70	58	9	3	2	0	73	77	
39	16	5	1	0	0	1	1	1	0	0	0	1	2	0	0	0	2	2	60	1	3	0	1	65	68	40	6	3	1	0	50	53
55	35	2	0	1	0	38	39	1	0	0	0	1	1	84	1	0	0	1	86	1	0	0	1	88	87	39	6	3	0	0	48	50
57	42	3	1	0	0	46	47	3	0	0	0	3	3	2	0	0	0	2	79	7	1	0	1	88	89	69	15	0	0	2	74	74
183	117	13	4	1	1	166	170	5	1	0	0	6	8	5	0	0	0	218	15	4	0	4	238	243	188	31	9	3	2	164	173	
51	53	8	2	0	1	64	66	0	0	0	0	0	0	0	0	0	0	59	8	2	0	3	72	76	47	6	1	0	0	54	55	
34	22	4	1	0	0	27	28	0	0	0	0	3	3	1	0	0	0	52	11	0	0	1	64	65	53	11	0	0	0	64	64	
28	29	8	0	0	0	37	37	1	0	0	0	1	1	64	6	1	0	1	64	6	1	0	1	64	66	37	8	2	0	0	47	48
43	45	3	0	0	0	48	48	4	0	0	0	4	3	0	0	0	0	59	7	0	0	0	66	66	41	5	0	0	0	46	46	
156	149	23	3	0	1	176	179	5	0	0	0	5	5	6	1	0	0	1	226	32	3	0	5	266	273	178	30	3	0	0	211	213
50	25	7	2	0	3	37	41	2	0	0	0	2	2	1	0	0	0	50	4	0	0	1	55	56	47	5	0	0	0	52	52	
35	28	8	1	0	0	37	38	1	0	0	0	1	5	1	0	0	0	47	4	2	0	1	54	56	44	4	0	0	0	48	48	
30	19	2	1	0	0	22	23	1	0	0	0	1	1	7	30	0	0	0	44	3	0	0	47	47	49	5	1	0	0	55	56	
39	30	2	0	0	0	32	32	1	1	0	0	2	2	17	45	0	0	0	49	0	0	0	50	51	52	10	0	0	1	63	64	
154	102	19	4	0	3	128	133	5	1	0	0	6	6	2	0	0	0	2	190	11	2	0	3	206	210	192	24	1	0	1	148	152
39	29	0	1	0	0	30	31	0	0	0	0	2	2	0	0	0	0	61	3	0	0	1	65	66	49	3	0	0	0	52	52	
24	22	3	0	0	0	25	25	3	0	0	0	3	3	18	15	1	0	0	44	2	0	0	1	47	48	35	2	0	0	1	38	39
30	11	4	1	0	0	16	17	0	0	0	0	0	5	2	0	0	0	2	61	3	0	0	64	64	40	2	0	0	0	42	42	
19	15	0	0	0	0	15	15	1	0	0	0	1	1	13	45	1	0	0	48	1	0	0	50	51	34	3	0	0	0	37	37	
112	77	7	2	0	0	86	87	4	0	0	0	4	4	9	7	0	0	9	17	0	0	0	17	17	229	158	10	0	0	1	969	970
1949	1842	339	40	13	8	1542	1597	64	4	0	0	68	68	82	10	1	0	0	94	56	3	0	4	63	67	2844	217	40	6	41	3148	2434

SITE:	02		22nd October 2019		02		22nd October 2019		DATE:		22nd October 2019		PCU's Through Junction				
	LOCATION:	Howth Road/Church Road	DATE:	22nd October 2019	SITE:	02	DATE:	22nd October 2019	LOCATION:	Howth Road/Church Road	DATE:	22nd October 2019					
LOCATION:	Howth Road/Church Road	DATE:	22nd October 2019	SITE:	02	DATE:	22nd October 2019	LOCATION:	Howth Road/Church Road	DATE:	22nd October 2019	PCU's Through Junction					
TIME	CAR	LGV	DGV	DGV2	BUS	TOT	PCU	TIME	CAR	LGV	DGV	DGV2	BUS	TOT	PCU	PCU	
07:00	29	6	0	0	0	35	0	07:00	0	0	0	0	0	0	0	65	106
07:15	35	14	1	0	1	51	2	07:15	1	1	0	0	0	1	1	61	136
07:30	37	7	3	0	2	49	5	07:30	1	0	0	0	0	1	1	69	137
07:45	54	11	2	2	0	69	6	07:45	0	0	0	0	0	2	2	58	146
H/TOT	155	38	6	2	3	204	13	H/TOT	2	1	0	0	0	4	4	253	525
08:00	38	17	2	1	2	60	6	08:00	2	0	0	0	0	3	3	72	193
08:15	56	9	1	0	2	68	34	08:15	3	0	0	0	0	5	5	73	245
08:30	118	14	4	0	1	137	25	08:30	5	0	0	0	0	7	7	109	353
08:45	78	9	4	0	1	92	14	08:45	1	0	0	0	0	13	14	98	264
H/TOT	290	49	11	1	6	357	95	H/TOT	11	0	0	0	0	28	29	352	1055
09:00	74	3	0	0	1	78	12	09:00	0	0	0	0	0	4	4	140	281
09:15	56	14	3	0	1	74	10	09:15	1	0	0	0	0	5	5	106	225
09:30	45	11	3	0	0	59	21	09:30	2	0	0	0	0	3	3	79	202
09:45	75	13	2	0	2	92	16	09:45	3	0	0	0	0	2	2	77	225
H/TOT	250	41	8	0	4	303	58	H/TOT	6	0	0	0	0	14	14	402	932
10:00	77	9	8	0	1	95	10	10:00	3	0	0	0	0	3	3	66	217
10:15	64	9	2	0	1	76	19	10:15	2	0	0	0	0	2	2	73	220
10:30	78	7	6	1	0	92	19	10:30	3	0	1	0	0	4	4	85	250
10:45	96	18	2	0	2	118	13	10:45	4	0	0	0	0	2	2	87	269
H/TOT	315	43	18	1	4	381	71	H/TOT	12	0	1	0	0	11	11	311	956
11:00	81	12	1	1	0	95	11	11:00	3	0	0	0	0	3	3	83	218
11:15	75	11	3	0	2	91	16	11:15	0	0	0	0	0	2	2	87	226
11:30	74	13	2	2	0	91	17	11:30	0	0	0	0	0	1	1	82	238
11:45	103	11	1	0	1	116	15	11:45	3	0	0	0	0	3	3	70	239
H/TOT	333	47	7	3	3	393	54	H/TOT	6	0	0	0	0	3	3	322	920
12:00	101	1	4	1	1	108	12	12:00	5	1	0	0	0	2	2	76	244
12:15	66	8	4	0	2	80	13	12:15	1	1	0	0	0	1	1	93	222
12:30	104	6	3	1	0	114	17	12:30	0	0	0	0	0	3	3	87	252
12:45	100	9	2	0	1	112	14	12:45	2	0	0	0	0	2	2	73	239
H/TOT	371	24	13	2	4	414	67	H/TOT	8	2	0	0	0	9	9	329	956

SITE:	03		22nd October 2019		SITE:		03		22nd October 2019		DATE:		22nd October 2019			PCU's Through Junction																	
LOCATION:	Howth Road/Offington Park		TuesDAY		LOCATION:		Howth Road/Offington Park		TuesDAY		DAY:		Tuesday																				
		MOVEMENT 1		MOVEMENT 2		MOVEMENT 3		MOVEMENT 4		MOVEMENT 5		MOVEMENT 6																					
TIME	CAR	DGV	DGV2	TOT	PCU	LGV	DGV	DGV2	TOT	PCU	CAR	LGV	DGV	DGV2	TOT	PCU																	
13:00	75	11	2	0	1	89	91	0	0	0	0	0	0	0	0	11	90	10	2	0	0	102	103										
13:15	85	10	1	0	1	97	99	1	0	0	0	0	0	0	0	18	19	66	4	3	1	2	76	81									
13:30	87	7	1	0	1	96	98	2	0	0	0	0	0	0	0	8	8	79	10	3	0	1	93	96									
13:45	85	9	0	0	0	94	94	8	0	0	0	0	0	0	0	9	9	77	4	3	1	0	85	88									
H/TOT	332	37	4	0	3	376	381	41	3	1	0	0	0	0	0	46	47	312	28	11	2	3	356	367									
14:00	77	9	1	0	2	89	92	14	0	0	0	0	0	0	0	10	7	95	9	0	0	1	105	106									
14:15	77	1	0	0	0	79	80	25	1	0	0	0	0	0	0	14	14	70	5	2	0	1	78	80									
14:30	65	6	3	2	2	78	84	10	0	0	0	0	0	0	0	9	9	85	7	1	0	1	94	96									
14:45	101	6	4	0	1	112	115	18	2	0	0	0	0	0	0	11	12	97	4	1	0	1	103	105									
H/TOT	320	22	9	2	5	358	370	67	3	0	0	0	0	0	0	41	42	347	25	4	0	4	380	386									
15:00	72	7	3	0	1	83	86	14	0	0	0	0	0	0	0	7	7	77	8	2	2	1	90	95									
15:15	79	6	3	0	0	88	90	12	1	0	0	0	0	0	0	9	9	80	5	5	0	0	90	93									
15:30	80	5	0	0	1	86	87	13	2	0	0	0	0	0	0	19	20	78	8	3	0	3	92	97									
15:45	72	8	2	0	0	82	83	6	1	0	0	0	0	0	0	11	11	78	9	0	0	1	88	89									
H/TOT	303	26	8	0	2	339	345	45	4	0	0	0	0	0	0	62	63	313	30	10	2	5	360	373									
16:00	82	13	0	0	2	97	99	16	0	0	0	0	0	0	0	8	8	78	10	2	0	2	92	95									
16:15	81	7	1	0	0	89	90	9	0	0	0	0	0	0	0	11	11	67	15	0	0	1	83	84									
16:30	84	8	0	0	1	93	94	12	0	0	0	0	0	0	0	6	6	64	13	2	0	1	80	82									
16:45	76	6	1	0	1	84	86	17	0	0	0	0	0	0	0	9	9	82	8	1	0	0	91	92									
H/TOT	323	34	2	0	4	363	368	54	0	0	0	1	0	0	0	34	34	291	46	5	0	4	346	353									
17:00	80	13	0	0	0	93	93	15	0	0	0	0	0	0	0	7	7	64	4	0	0	1	69	70									
17:15	68	5	1	0	1	75	77	14	1	0	0	0	0	0	0	8	8	65	7	3	0	1	76	79									
17:30	92	3	1	0	1	97	99	12	0	0	0	0	0	0	0	5	5	60	4	1	0	0	65	66									
17:45	89	6	0	0	1	96	97	12	0	0	0	0	0	0	0	10	10	79	8	1	0	1	89	91									
H/TOT	329	27	2	0	3	361	365	53	1	0	0	0	0	0	0	31	31	268	23	5	0	3	299	305									
18:00	80	5	0	0	0	85	85	14	0	0	0	0	0	0	0	8	8	60	3	1	0	2	66	69									
18:15	103	2	0	0	0	105	105	21	0	1	0	0	0	0	0	6	6	61	4	0	0	2	67	69									
18:30	124	8	1	0	1	134	136	10	0	0	0	0	0	0	0	11	11	22	0	0	0	0	64	64									
18:45	97	4	1	0	0	102	103	5	0	0	0	0	0	0	0	6	6	19	85	1	0	0	1	87	88								
H/TOT	404	19	2	0	1	426	428	50	0	1	0	0	0	0	0	30	30	59	267	11	1	0	5	284	290								
P/TOT	3566	403	80	11	39	4099	4182	479	26	5	0	6	516	525	466	33	5	0	0	0	0	2	620	625	484	21	6	0	3	514	514	4003	4109

SITE:	04				22nd October 2019				04				22nd October 2019				PCU's Through Junction							
	LOCATION:	Harbour Road/Church Street	DATE:	22nd October 2019	SITE:	04	DATE:	22nd October 2019	LOCATION:	Harbour Road/Church Street	DATE:	04	DATE:	22nd October 2019										
LOCATION:	Harbour Road/Church Street	DATE:	Tuesday	LOCATION:	Harbour Road/Church Street	DATE:	Tuesday	LOCATION:	Harbour Road/Church Street	DATE:	Tuesday	LOCATION:	Harbour Road/Church Street	DATE:	Tuesday									
TIME	CAR	LGV	DGV	DGV2	BUS	TOT	PCU	TIME	CAR	LGV	DGV	DGV2	BUS	TOT	PCU	TIME	CAR	LGV	DGV	DGV2	BUS	TOT	PCU	
07:00	7	1	0	0	1	9	10	07:00	0	0	0	0	0	0	0	0	07:00	0	0	0	0	0	0	0
07:15	17	9	0	0	0	26	9	07:15	1	0	0	0	0	1	1	1	07:15	1	0	0	0	0	0	0
07:30	31	7	1	0	1	40	18	07:30	0	0	0	0	0	0	0	0	07:30	0	0	0	0	0	0	0
07:45	36	10	1	0	1	48	20	07:45	1	0	0	0	0	1	1	1	07:45	1	0	0	0	0	0	0
H/TOT	91	27	2	0	3	123	60	H/TOT	2	0	0	0	0	2	2	1	H/TOT	2	0	0	0	0	1	200
08:00	16	6	0	0	3	25	28	08:00	1	0	0	0	0	1	1	0	08:00	1	0	0	0	0	0	60
08:15	19	4	1	0	2	26	29	08:15	2	0	0	0	0	2	2	1	08:15	2	0	0	0	0	1	57
08:30	42	1	1	0	0	44	45	08:30	0	1	0	0	0	1	1	1	08:30	0	1	0	0	0	2	77
08:45	45	4	0	0	1	50	51	08:45	0	0	0	0	0	0	0	2	08:45	0	0	0	0	0	2	85
H/TOT	122	15	2	0	6	145	152	H/TOT	3	1	0	0	0	4	4	4	H/TOT	3	1	0	0	0	5	279
09:00	46	7	2	0	1	56	24	09:00	3	0	0	0	0	3	3	0	09:00	3	0	0	0	0	0	78
09:15	37	7	0	0	1	45	17	09:15	2	0	0	0	0	2	0	0	09:15	2	0	0	0	0	1	64
09:30	24	5	0	0	0	29	11	09:30	0	0	0	0	0	0	0	0	09:30	0	0	0	0	0	0	55
09:45	33	4	1	0	1	39	16	09:45	1	0	0	0	0	1	1	0	09:45	1	0	0	0	0	1	48
H/TOT	140	23	3	0	3	169	74	H/TOT	4	0	0	0	0	4	4	0	H/TOT	4	0	0	0	0	2	245
10:00	45	3	1	0	2	51	16	10:00	2	0	0	0	0	2	2	1	10:00	2	0	0	0	0	1	53
10:15	29	4	2	0	1	36	18	10:15	0	0	0	0	0	0	0	0	10:15	0	0	0	0	0	2	48
10:30	45	5	4	0	0	54	14	10:30	0	0	0	0	0	0	0	0	10:30	0	0	0	0	0	2	58
10:45	60	3	1	0	0	64	23	10:45	0	0	0	0	0	0	0	1	10:45	0	0	0	0	0	1	60
H/TOT	179	15	8	0	3	205	74	H/TOT	2	0	0	0	0	2	2	4	H/TOT	2	0	0	0	0	7	219
11:00	34	4	2	0	2	42	19	11:00	0	0	0	0	0	0	0	0	11:00	0	0	0	0	0	1	57
11:15	40	5	1	0	0	46	11	11:15	0	0	0	0	0	0	0	0	11:15	0	0	0	0	0	2	54
11:30	60	6	3	0	1	70	14	11:30	0	0	0	0	0	0	0	0	11:30	0	0	0	0	0	1	60
11:45	51	1	0	0	1	53	26	11:45	0	0	0	0	0	0	0	2	11:45	0	0	0	0	0	4	61
H/TOT	185	16	6	0	4	211	70	H/TOT	0	0	0	0	0	0	0	5	H/TOT	0	0	0	0	0	8	232
12:00	48	4	1	0	1	54	27	12:00	1	0	0	0	0	1	1	0	12:00	1	0	0	0	0	0	58
12:15	42	6	1	0	2	51	21	12:15	0	0	0	0	0	0	0	1	12:15	0	0	0	0	0	1	63
12:30	51	0	2	0	0	53	26	12:30	1	0	0	0	0	1	1	0	12:30	1	0	0	0	0	0	60
12:45	57	3	0	0	0	60	31	12:45	1	0	0	0	0	1	1	2	12:45	1	0	0	0	0	2	48
H/TOT	198	13	4	0	3	218	105	H/TOT	3	0	0	0	0	3	3	3	H/TOT	3	0	0	0	0	3	229

SITE:	04	DATE:	22nd October 2019	SITE:	04	DATE:	22nd October 2019	PCU's Through Junction																															
LOCATION:	Harbour Road/Church Street	LOCATION:	Harbour Road/Church Street	DATE:	04	DATE:	22nd October 2019																																
DAY:	Tuesday	DAY:	Tuesday	MOVEMENT 1	MOVEMENT 2	MOVEMENT 3	MOVEMENT 4	MOVEMENT 5	MOVEMENT 6	PCU																													
TIME	CAR	LG	OGV	DGV	BUS	TOT	PCU	CAR	LG	OGV	DGV	BUS	TOT	PCU	PCU																								
13:00	38	8	2	0	2	50	43	18	2	0	0	0	20	20	13:00	20	13:00	0	0	0	0	0	0	0	2	2	45	3	1	0	1	50	52	169					
13:15	52	6	0	0	1	59	60	32	1	1	0	0	34	35	16	2	2	0	0	0	0	1	1	3	0	0	0	0	0	0	3	42	3	1	0	1	47	49	168
13:30	59	3	1	0	1	64	66	20	3	1	0	0	24	25	19	1	0	0	0	0	2	2	1	0	0	0	1	1	51	6	2	0	1	60	62	175			
13:45	54	5	0	0	0	59	59	26	1	0	0	0	27	27	16	1	2	0	0	0	0	0	0	0	0	0	2	2	53	5	0	0	1	59	60	168			
H/TOT	203	22	3	0	4	232	238	116	8	3	0	0	127	129	69	6	4	0	0	0	3	3	8	0	0	0	8	8	191	17	4	0	4	216	222	680			
14:00	45	3	1	0	2	51	54	30	3	0	0	0	33	33	33	3	0	0	0	0	1	1	0	0	0	0	0	0	62	4	0	0	0	66	66	190			
14:15	35	1	1	0	0	37	38	35	4	0	0	0	39	39	10	1	1	0	0	0	0	0	4	0	0	0	4	4	50	7	0	0	1	58	59	152			
14:30	50	3	1	0	1	55	57	26	1	0	0	1	28	29	31	2	0	0	0	0	0	0	2	0	0	0	2	2	64	4	1	0	1	70	72	192			
14:45	58	5	3	0	0	66	68	33	3	2	0	0	38	39	16	1	1	0	0	0	1	1	0	0	0	1	1	0	45	3	1	0	2	51	54	180			
H/TOT	188	12	6	0	3	209	215	124	11	2	0	1	138	140	90	7	2	0	0	0	2	2	6	0	0	0	6	6	221	18	2	0	4	245	250	713			
15:00	45	1	1	0	2	49	52	24	2	2	0	0	28	29	16	2	0	0	0	0	0	0	1	0	0	0	0	1	55	7	1	0	1	64	66	165			
15:15	55	4	2	0	2	63	66	23	1	1	0	0	25	26	18	0	1	0	0	0	2	2	0	0	0	0	2	2	0	0	0	0	45	47	160				
15:30	63	3	0	0	1	67	68	31	2	0	0	0	33	33	30	0	2	0	0	1	1	2	0	0	0	2	2	49	6	0	0	2	57	59	198				
15:45	62	1	0	0	0	63	63	28	3	1	0	0	32	33	20	1	0	0	0	0	0	0	3	0	0	0	3	3	47	4	0	0	2	53	55	175			
H/TOT	225	9	3	0	5	242	249	106	8	4	0	0	118	120	84	3	3	0	0	0	3	3	6	0	0	0	6	6	190	20	3	1	5	219	227	698			
16:00	43	6	0	0	1	50	51	25	5	0	0	0	30	30	18	2	1	0	0	0	1	1	3	0	0	0	3	3	45	7	1	0	1	54	56	162			
16:15	54	3	0	0	1	58	59	31	3	0	0	0	34	34	15	1	0	0	0	0	0	0	0	0	0	0	0	0	63	8	0	0	1	72	73	182			
16:30	58	5	1	0	1	65	67	37	3	0	0	0	40	40	16	1	0	0	0	1	1	3	0	0	0	3	3	42	7	2	0	1	52	54	182				
16:45	54	6	2	0	0	62	63	30	2	1	0	0	33	34	24	2	0	0	0	0	1	1	1	0	0	0	1	1	60	7	1	0	0	68	69	193			
H/TOT	209	20	3	0	3	235	240	123	13	1	0	0	137	138	73	6	1	0	0	0	3	3	7	0	0	0	7	7	210	29	4	0	3	246	251	719			
17:00	55	5	0	0	0	60	60	38	3	0	0	0	41	41	16	1	0	0	0	0	1	1	2	0	0	0	2	2	40	4	0	0	1	45	46	167			
17:15	43	3	1	0	1	48	50	24	5	1	0	0	30	31	10	0	0	0	0	0	0	0	0	0	0	0	0	0	45	2	2	0	2	51	54	144			
17:30	54	7	0	0	1	62	63	54	1	0	0	0	55	55	16	0	0	0	0	0	0	0	2	0	0	0	2	2	42	2	0	0	44	44	180				
17:45	53	6	0	0	1	60	61	48	2	0	0	0	50	50	18	2	0	0	0	0	1	1	4	0	0	0	4	4	59	6	0	0	1	66	67	203			
H/TOT	205	21	1	0	3	230	234	164	11	1	0	0	176	177	60	3	0	0	0	0	2	2	8	0	0	0	8	8	186	14	2	0	4	206	211	694			
18:00	53	3	0	0	1	57	58	41	2	0	0	1	44	45	15	2	0	0	0	0	0	0	3	0	0	0	3	3	48	2	0	0	1	51	52	175			
18:15	60	2	0	0	0	62	62	44	0	0	0	0	44	44	19	0	0	0	0	0	0	0	3	0	0	0	3	3	33	1	0	0	2	36	38	168			
18:30	75	4	0	0	0	79	79	49	1	0	0	0	50	50	21	0	0	0	0	0	1	1	2	0	0	0	2	2	67	2	0	0	69	69	222				
18:45	66	2	1	0	1	70	72	38	6	0	0	0	44	44	24	1	0	0	0	0	0	0	5	0	0	0	5	5	82	0	0	0	1	83	84	210			
H/TOT	254	11	1	0	2	268	271	172	9	0	0	1	182	183	79	3	0	0	0	0	1	1	13	0	0	0	13	13	210	5	0	0	4	219	223	775			
P/TOT	2189	204	42	0	42	2487	2550	1303	107	26	1	3	1440	1457	1004	59	19	1	3	1086	1100	28	1	0	0	0	29	29	65	8	1	0	0	43	2755	2823	8034		

APPENDIX

3

FLOW
DIAGRAMS

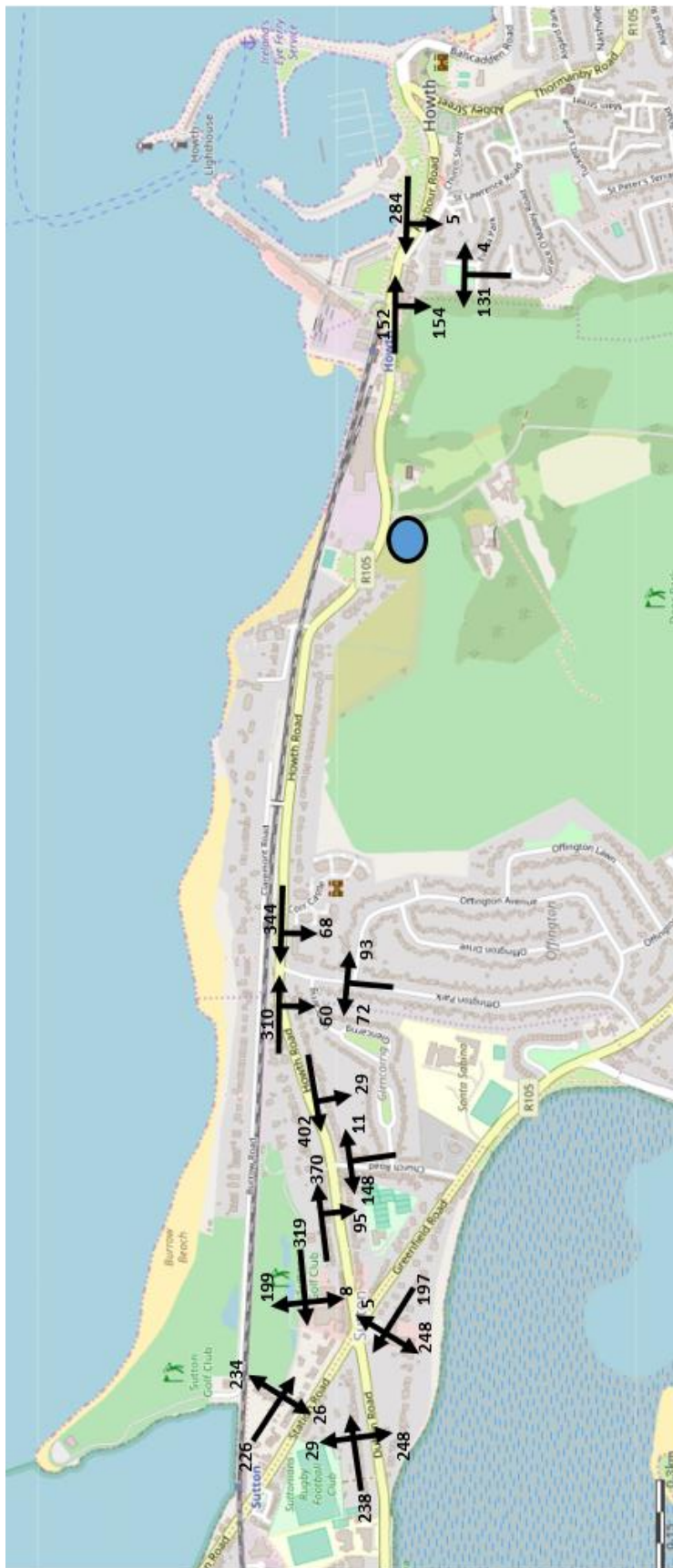


DIAGRAM 1: NETWORK FLOWS AM PEAK (OCTOBER 2019 SURVEY)



DIAGRAM 2: NETWORK FLOWS PM PEAK (OCTOBER 2019 SURVEY)



DIAGRAM 3: ADJACENT DEVELOPMENT FLOWS AM PEAK



DIAGRAM 4: ADJACENT DEVELOPMENT FLOWS PM PEAK



DIAGRAM 5: PROPOSED DEVELOPMENT FLOWS AM PEAK



DIAGRAM 6: PROPOSED DEVELOPMENT FLOWS PM PEAK

4

DETAILED JUNCTION ANALYSIS

APPENDIX 4A

SUTTON CROSS SIGNALISED JUNCTION

<h1>Junctions 9</h1>
<h2>OSCADY 9 - Signalised Intersection Module</h2>
For sales and distribution information, program advice and maintenance, contact TRL: +44 (0)1344 379777 software@trl.co.uk www.trlsoftware.co.uk
The users of this computer program for the solution of an engineering problem are in no way relieved of their responsibility for the correctness of the solution

Filename: Sutton Cross 2020 exist latest.j9
Path: C:\Users\Martin.Rogers\Documents\howth road
Report generation date: 12/04/2020 17:09:04

»2020, AM exist
 »2020, PM exist

Summary of junction performance

	AM exist				PM exist			
	Queue (PCU)	Delay (s)	DOS	LOS	Queue (PCU)	Delay (s)	DOS	LOS
2020								
Arm A	24.7	67.69	0.79	E	16.5	47.79	0.61	D
Arm B	14.0	49.68	0.70	D	16.1	62.13	0.81	E
Arm C	23.7	76.87	0.95	E	15.3	55.52	0.57	E
Arm D	14.4	28.74	0.45	C	9.1	22.57	0.29	C

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle.

File summary

File Description

Title	Sutton Cross
Location	Howth
Site number	
Date	10/04/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Min	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	DOS Threshold	Average Delay threshold (s)	Queue threshold (PCU)
		0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2020	AM exist	DIRECT	08:00	09:00	60	15
D2	2020	PM exist	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2020, AM exist

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		56.05	E

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description
A	Howth Road	
B	Carrickrock Road	
C	Dublin Road	
D	Station Road	

OSCADY Traffic Streams

Arm	Traffic Stream	Phase	Notional EEG (s)	Signals EEG (s)	Destination arms	Straight move
A	1	E	0.00	0.00	B, C	C
	2	D	0.00	0.00	D	C
B	1	F	0.00	0.00	A, C, D	D
C	1	B	0.00	1.00	A	A
	2	A	0.00	0.00	B	A
D	1	G	0.00	0.00	A	B
	2	C	0.00	5.00	B, C	B

OSCADY Lanes

Arm	Traffic Stream	Destination arms	Gradient (%)	Width (m)	Turning radius (m)	Nearside lane	Has bay
A	1	B, C	0	2.50	5.00	✓	
	2	D	0	2.50	5.00	✓	
B	1	A, C, D	0	3.00	10.00	✓	
C	1	A	0	2.50		✓	
	2	B	0	2.50	10.00	✓	
D	1	A	0	2.50	5.00	✓	
	2	B, C	0	2.50	5.00	✓	

Signal Timings

Junction 1

Junction	Sequence to use	Cycle time (s)	Maximum cycle time (s)	Start displacement (s)	End displacement (s)
1	1	160	300	1.40	2.90

Optimisation options

Junction	Optimise stage lengths	Optimise cycle time	Optimiser demand source	Optimiser message
1	✓		Average	Timings provide delay minimisation.

Phases

Junction	Phase	Name	Minimum green (s)
1	A		15
	B		15
	C		15
	D		15
	E		15
	F		15
	G		15

Library Stages

Junction	Library Stage	Phases in stage	User stage minimum (s)	Run every N cycles	Probability of running (%)
1	1	B, E	7		
	2	D, E, G	7		
	3	C, G, F	7		
	4		7		
	5	A, B	7		

Losing / Gaining Phase Delays

Junction	Delay	Type	Phase	From stage	To stage	Relative delay (s)
1	1	Losing	B	1	2	1

Stage Sequences

Junction	Sequence	Name	Stage IDs	Stage ends
1	1		1, 2, 3, 4, 5	12, 52, 116, 124, 0

Intergreen Matrix for Junction 1

		To						
		A	B	C	D	E	F	G
From	A			5	5	5	5	5
	B			5	5		5	5
	C	0	0		0	5		
	D	0	0	0			5	

	E	0		0			0	
	F	0	0		0	5		
	G	0	0					

Interstage Matrix for Junction 1

		To				
		1	2	3	4	5
From	1	0	6	5	0	0
	2	0	0	5	0	0
	3	5	5	0	0	0
	4	0	0	0	0	0
	5	5	5	5	0	0

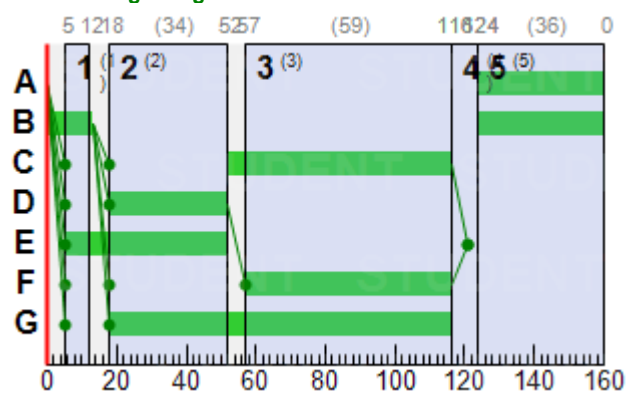
Resultant Stages

Junction	Resultant Stage	Library Stage ID	Phases in this stage	Stage start (s)	Stage end (s)	Stage duration (s)	User stage minimum (s)	Stage minimum (s)
1	1	1	B,E	5	12	7	7	7
	2	2	D,E,G	18	52	34	7	15
	3	3	C,G,F	57	116	59	7	15
	4	4		116	124	8	7	7
	5	5	A,B	124	0	36	7	15

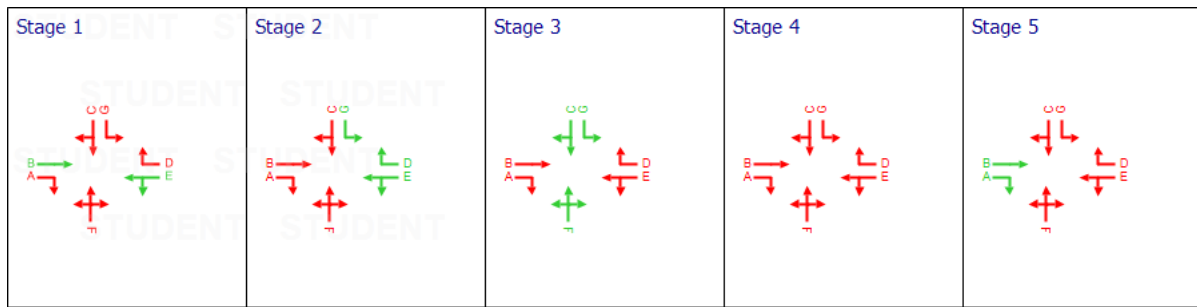
Resultant Phase Green Periods

Junction	Phase	Green period	Start time (s)	End time (s)	Duration (s)
1	A	1	124	0	36
	B	1	124	13	49
	C	1	52	116	64
	D	1	18	52	34
	E	1	5	52	47
	F	1	57	116	59
	G	1	18	116	98

Phase Timings Diagram for Junction 1



Stage Sequence Diagram for Junction 1



Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2020	AM exist	DIRECT	08:00	09:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:00 - 08:15

		To			
		A	B	C	D
From	A	0.00	0.00	55.00	37.00
	B	0.00	0.00	49.00	59.00
	C	49.00	48.00	0.00	0.00
	D	48.00	58.00	4.00	0.00

Demand (PCU/TS)

08:15 - 08:30

		To			
		A	B	C	D
From	A	0.00	0.00	68.00	45.00
	B	2.00	0.00	69.00	44.00
	C	54.00	90.00	0.00	0.00
	D	43.00	73.00	10.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	D
08:30 - 08:45	From				
	A	0.00	1.00	105.00	63.00
	B	2.00	0.00	67.00	41.00
	C	77.00	58.00	0.00	0.00
	D	95.00	67.00	6.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	D
08:45 - 09:00	From				
	A	0.00	0.00	92.00	50.00
	B	1.00	0.00	63.00	54.00
	C	59.00	53.00	0.00	0.00
	D	49.00	28.00	6.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:00-08:15	A	92.00	92.00
	B	108.00	108.00
	C	97.00	97.00
	D	110.00	110.00
08:15-08:30	A	113.00	113.00
	B	115.00	115.00
	C	144.00	144.00
	D	126.00	126.00
08:30-08:45	A	169.00	169.00
	B	110.00	110.00
	C	135.00	135.00
	D	168.00	168.00
08:45-09:00	A	142.00	142.00
	B	118.00	118.00
	C	112.00	112.00
	D	83.00	83.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	0.79	67.69	24.7	E
B	0.70	49.68	14.0	D
C	0.95	76.87	23.7	E
D	0.45	28.74	14.4	C

Main Results for each time segment**08:00 - 08:15**

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	55.00	466.25	48.50	0.00	141.33	0.389	48.04	7.0	46.447	D
	2	37.00	358.65	35.50	0.00	79.58	0.465	31.64	5.4	59.697	E
B	1	108.00	448.24	60.50	0.00	169.49	0.637	95.40	12.6	46.110	D
C	1	49.00	466.25	50.50	0.00	147.16	0.333	42.94	6.1	43.683	D
	2	48.00	405.43	37.50	0.00	95.02	0.505	41.16	6.8	58.779	E
D	1	48.00	358.65	99.50	0.00	223.04	0.215	44.74	3.3	13.867	B
	2	62.00	457.40	65.50	0.00	187.25	0.331	55.39	6.6	33.699	C

08:15 - 08:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	68.00	466.25	48.50	0.00	141.33	0.481	66.27	8.7	48.972	D
	2	45.00	358.65	35.50	0.00	79.58	0.565	43.70	6.7	63.850	E
B	1	115.00	438.17	60.50	0.00	165.68	0.694	113.97	13.6	49.085	D
C	1	54.00	466.25	50.50	0.00	147.16	0.367	53.36	6.7	44.494	D
	2	90.00	405.43	37.50	0.00	95.02	0.947	79.86	17.0	96.294	F
D	1	43.00	358.65	99.50	0.00	223.04	0.193	43.34	2.9	13.576	B
	2	83.00	449.99	65.50	0.00	184.21	0.451	80.67	8.9	36.595	D

08:30 - 08:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	106.00	464.93	48.50	0.00	140.93	0.752	100.27	14.4	60.764	E
	2	63.00	358.65	35.50	0.00	79.58	0.792	59.37	10.3	79.343	E
B	1	110.00	437.58	60.50	0.00	165.46	0.665	110.66	13.0	47.910	D
C	1	77.00	466.25	50.50	0.00	147.16	0.523	73.99	9.7	48.818	D
	2	58.00	405.43	37.50	0.00	95.02	0.610	66.49	8.5	67.736	E
D	1	95.00	358.65	99.50	0.00	223.04	0.426	91.34	6.6	17.338	B
	2	73.00	455.03	65.50	0.00	186.28	0.392	74.12	7.8	35.122	D

08:45 - 09:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	92.00	466.25	48.50	0.00	141.33	0.651	94.27	12.1	55.896	E

	2	50.00	358.65	35.50	0.00	79.58	0.628	52.70	7.6	68.845	E
B	1	118.00	442.73	60.50	0.00	167.41	0.705	116.95	14.0	49.682	D
C	1	59.00	466.25	50.50	0.00	147.16	0.401	61.37	7.3	45.389	D
	2	53.00	405.43	37.50	0.00	95.02	0.558	53.84	7.6	61.343	E
D	1	49.00	358.65	99.50	0.00	223.04	0.220	52.24	3.3	13.933	B
	2	34.00	442.81	65.50	0.00	181.27	0.188	38.22	3.6	30.919	C

2020, PM exist

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		47.80	D

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2020	PM exist	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

		To				
		A	B	C	D	
16:00 - 16:15	From	A	0.00	0.00	76.00	55.00
		B	0.00	0.00	53.00	66.00
		C	65.00	38.00	0.00	0.00
		D	44.00	25.00	2.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:15 - 16:30	From	A	0.00	4.00	65.00	64.00
		B	0.00	0.00	34.00	28.00
		C	74.00	39.00	0.00	0.00
		D	57.00	17.00	4.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:30 - 16:45	From	A	0.00	0.00	66.00	48.00
		B	1.00	0.00	30.00	37.00
		C	60.00	50.00	0.00	0.00
		D	42.00	30.00	0.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:45 - 17:00	From	A	0.00	3.00	66.00	46.00
		B	4.00	0.00	43.00	48.00
		C	70.00	40.00	0.00	0.00
		D	67.00	39.00	2.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:00-16:15	A	131.00	131.00
	B	119.00	119.00
	C	103.00	103.00

	D	71.00	71.00
16:15-16:30	A	133.00	133.00
	B	62.00	62.00
	C	113.00	113.00
	D	78.00	78.00
16:30-16:45	A	114.00	114.00
	B	68.00	68.00
	C	110.00	110.00
	D	72.00	72.00
16:45-17:00	A	115.00	115.00
	B	95.00	95.00
	C	110.00	110.00
	D	108.00	108.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	0.61	47.79	16.5	D
B	0.81	62.13	16.1	E
C	0.57	55.52	15.3	E
D	0.29	22.57	9.1	C

Main Results for each time segment

16:00 - 16:15

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	76.00	466.25	59.50	0.00	173.39	0.438	67.31	8.7	40.105	D
	2	55.00	358.65	46.50	0.00	104.23	0.528	47.72	7.3	53.117	D
B	1	119.00	448.77	52.50	0.00	147.25	0.808	102.93	16.1	62.133	E
C	1	65.00	466.25	47.50	0.00	138.42	0.470	56.63	8.4	49.338	D
	2	38.00	405.43	34.50	0.00	87.42	0.435	32.50	5.5	58.926	E
D	1	44.00	358.65	102.50	0.00	229.76	0.192	41.16	2.8	12.332	B
	2	27.00	456.11	57.50	0.00	163.92	0.165	23.91	3.1	35.545	D

16:15 - 16:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	69.00	458.28	59.50	0.00	170.42	0.405	69.82	7.9	39.326	D
	2	64.00	358.65	46.50	0.00	104.23	0.614	62.64	8.6	56.916	E
B	1	62.00	442.36	52.50	0.00	145.15	0.427	70.47	7.6	45.059	D
C	1	74.00	466.25	47.50	0.00	138.42	0.535	72.76	9.6	51.416	D
	2	39.00	405.43	34.50	0.00	87.42	0.446	38.85	5.7	59.407	E
D	1	57.00	358.65	102.50	0.00	229.76	0.248	56.15	3.7	13.058	B
	2	21.00	441.05	57.50	0.00	158.50	0.132	21.69	2.4	34.995	C

16:30 - 16:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	66.00	466.25	59.50	0.00	173.39	0.381	66.36	7.5	38.690	D
	2	48.00	358.65	46.50	0.00	104.23	0.461	50.35	6.3	51.028	D
B	1	68.00	448.11	52.50	0.00	147.04	0.462	67.24	8.4	45.710	D
C	1	60.00	466.25	47.50	0.00	138.42	0.433	61.91	7.7	48.427	D
	2	50.00	405.43	34.50	0.00	87.42	0.572	48.23	7.4	64.038	E
D	1	42.00	358.65	102.50	0.00	229.76	0.183	42.98	2.7	12.230	B
	2	30.00	466.25	57.50	0.00	167.56	0.179	28.96	3.4	35.791	D

16:45 - 17:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	69.00	460.25	59.50	0.00	171.15	0.403	68.64	7.9	39.253	D
	2	46.00	358.65	46.50	0.00	104.23	0.441	46.28	6.0	50.302	D
B	1	95.00	445.68	52.50	0.00	146.24	0.650	91.31	12.0	52.420	D
C	1	70.00	466.25	47.50	0.00	138.42	0.506	68.64	9.1	50.467	D
	2	40.00	405.43	34.50	0.00	87.42	0.458	41.60	5.8	59.978	E
D	1	67.00	358.65	102.50	0.00	229.76	0.292	65.36	4.4	13.669	B
	2	41.00	459.53	57.50	0.00	165.14	0.248	39.72	4.7	37.124	D

<h1>Junctions 9</h1>
<h2>OSCADY 9 - Signalised Intersection Module</h2>
For sales and distribution information, program advice and maintenance, contact TRL: +44 (0)1344 379777 software@trl.co.uk www.trlsoftware.co.uk
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Filename: Sutton Cross 2023 WOD.j9
Path: C:\Users\Martin.Rogers\Documents\howth road
Report generation date: 13/04/2020 16:43:36

»2023, AM WOD

»2023, PM WOD

Summary of junction performance

	AM WOD				PM WOD			
	Queue (PCU)	Delay (s)	DOS	LOS	Queue (PCU)	Delay (s)	DOS	LOS
2023								
Arm A	26.7	71.46	0.83	E	17.3	48.74	0.64	D
Arm B	14.6	51.14	0.73	D	17.2	65.30	0.84	E
Arm C	26.2	82.45	0.99	F	15.9	56.29	0.59	E
Arm D	15.0	29.00	0.47	C	9.9	23.38	0.30	C

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle.

File summary

File Description

Title	Sutton Cross
Location	Howth
Site number	
Date	10/04/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Min	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	DOS Threshold	Average Delay threshold (s)	Queue threshold (PCU)
		0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM WOD	DIRECT	08:00	09:00	60	15
D2	2023	PM WOD	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2023, AM WOD

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		58.88	E

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description
A	Howth Road	
B	Carrickrock Road	
C	Dublin Road	
D	Station Road	

OSCADY Traffic Streams

Arm	Traffic Stream	Phase	Notional EEG (s)	Signals EEG (s)	Destination arms	Straight move
A	1	E	0.00	0.00	B, C	C
	2	D	0.00	0.00	D	C
B	1	F	0.00	0.00	A, C, D	D
C	1	B	0.00	1.00	A	A
	2	A	0.00	0.00	B	A
D	1	G	0.00	0.00	A	B
	2	C	0.00	5.00	B, C	B

OSCADY Lanes

Arm	Traffic Stream	Destination arms	Gradient (%)	Width (m)	Turning radius (m)	Nearside lane	Has bay
A	1	B, C	0	2.50	5.00	✓	
	2	D	0	2.50	5.00	✓	
B	1	A, C, D	0	3.00	10.00	✓	
C	1	A	0	2.50		✓	
	2	B	0	2.50	10.00	✓	
D	1	A	0	2.50	5.00	✓	
	2	B, C	0	2.50	5.00	✓	

Signal Timings

Junction 1

Junction	Sequence to use	Cycle time (s)	Maximum cycle time (s)	Start displacement (s)	End displacement (s)
1	1	160	300	1.40	2.90

Optimisation options

Junction	Optimise stage lengths	Optimise cycle time	Optimiser demand source	Optimiser message
1	✓		Average	Timings provide delay minimisation.

Phases

Junction	Phase	Name	Minimum green (s)
1	A		15
	B		15
	C		15
	D		15
	E		15
	F		15
	G		15

Library Stages

Junction	Library Stage	Phases in stage	User stage minimum (s)	Run every N cycles	Probability of running (%)
1	1	B, E	7		
	2	D, E, G	7		
	3	C, G, F	7		
	4		7		
	5	A, B	7		

Losing / Gaining Phase Delays

Junction	Delay	Type	Phase	From stage	To stage	Relative delay (s)
1	1	Losing	B	1	2	1

Stage Sequences

Junction	Sequence	Name	Stage IDs	Stage ends
1	1		1, 2, 3, 4, 5	12, 52, 116, 124, 0

Intergreen Matrix for Junction 1

		To						
		A	B	C	D	E	F	G
From	A			5	5	5	5	5
	B			5	5		5	5
	C	0	0		0	5		
	D	0	0	0			5	

	E	0		0			0	
	F	0	0		0	5		
	G	0	0					

Interstage Matrix for Junction 1

		To				
		1	2	3	4	5
From	1	0	6	5	0	0
	2	0	0	5	0	0
	3	5	5	0	0	0
	4	0	0	0	0	0
	5	5	5	5	0	0

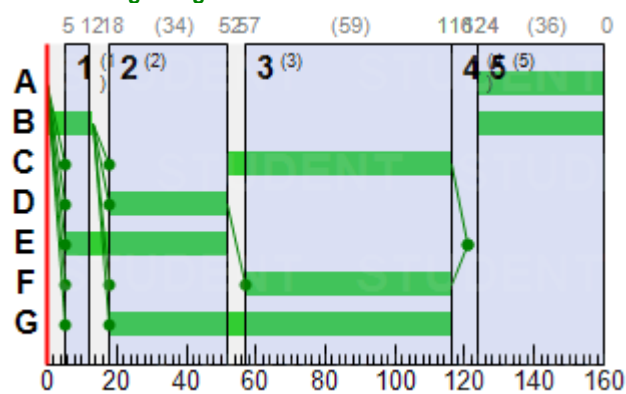
Resultant Stages

Junction	Resultant Stage	Library Stage ID	Phases in this stage	Stage start (s)	Stage end (s)	Stage duration (s)	User stage minimum (s)	Stage minimum (s)
1	1	1	B,E	5	12	7	7	7
	2	2	D,E,G	18	52	34	7	15
	3	3	C,G,F	57	116	59	7	15
	4	4		116	124	8	7	7
	5	5	A,B	124	0	36	7	15

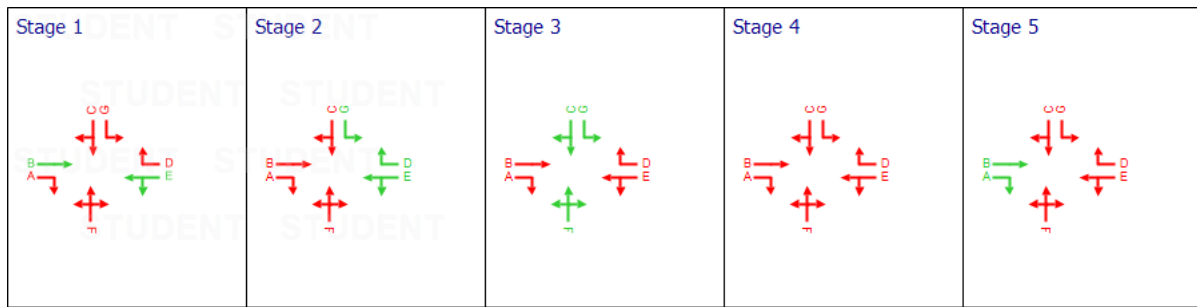
Resultant Phase Green Periods

Junction	Phase	Green period	Start time (s)	End time (s)	Duration (s)
1	A	1	124	0	36
	B	1	124	13	49
	C	1	52	116	64
	D	1	18	52	34
	E	1	5	52	47
	F	1	57	116	59
	G	1	18	116	98

Phase Timings Diagram for Junction 1



Stage Sequence Diagram for Junction 1



Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM WOD	DIRECT	08:00	09:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:00 - 08:15

		To			
		A	B	C	D
From	A	0.00	0.00	57.00	39.00
	B	0.00	0.00	51.00	61.00
	C	51.00	50.00	0.00	0.00
	D	50.00	60.00	4.00	0.00

Demand (PCU/TS)

08:15 - 08:30

		To			
		A	B	C	D
From	A	0.00	0.00	71.00	47.00
	B	2.00	0.00	72.00	46.00
	C	56.00	94.00	0.00	0.00
	D	45.00	76.00	10.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
08:30 - 08:45	From	A	0.00	3.00	109.00	66.00
		B	2.00	0.00	70.00	43.00
		C	80.00	60.00	0.00	0.00
		D	99.00	70.00	6.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
08:45 - 09:00	From	A	0.00	0.00	95.00	52.00
		B	1.00	0.00	65.00	56.00
		C	62.00	55.00	0.00	0.00
		D	51.00	29.00	6.00	0.00

Vehicle Mix**Heavy Vehicle Percentages**

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data**Demand for each time segment**

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:00-08:15	A	96.00	96.00
	B	112.00	112.00
	C	101.00	101.00
	D	114.00	114.00
08:15-08:30	A	118.00	118.00
	B	120.00	120.00
	C	150.00	150.00
	D	131.00	131.00
08:30-08:45	A	178.00	178.00
	B	115.00	115.00
	C	140.00	140.00
	D	175.00	175.00
08:45-09:00	A	147.00	147.00
	B	122.00	122.00
	C	117.00	117.00
	D	86.00	86.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	0.83	71.46	26.7	E
B	0.73	51.14	14.6	D
C	0.99	82.45	26.2	F
D	0.47	29.00	15.0	C

Main Results for each time segment**08:00 - 08:15**

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	57.00	466.25	48.50	0.00	141.33	0.403	49.78	7.2	46.806	D
	2	39.00	358.65	35.50	0.00	79.58	0.490	33.33	5.7	60.607	E
B	1	112.00	448.14	60.50	0.00	169.45	0.661	98.86	13.1	47.146	D
C	1	51.00	466.25	50.50	0.00	147.16	0.347	44.69	6.3	43.997	D
	2	50.00	405.43	37.50	0.00	95.02	0.526	42.85	7.1	59.543	E
D	1	50.00	358.65	99.50	0.00	223.04	0.224	46.60	3.4	13.988	B
	2	64.00	457.67	65.50	0.00	187.36	0.342	57.17	6.8	33.927	C

08:15 - 08:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	71.00	466.25	48.50	0.00	141.33	0.502	69.13	9.1	49.618	D
	2	47.00	358.65	35.50	0.00	79.58	0.591	45.68	7.0	65.068	E
B	1	120.00	438.22	60.50	0.00	165.70	0.724	118.77	14.4	50.770	D
C	1	56.00	466.25	50.50	0.00	147.16	0.381	55.36	7.0	44.826	D
	2	94.00	405.43	37.50	0.00	95.02	0.989	81.87	19.3	104.863	F
D	1	45.00	358.65	99.50	0.00	223.04	0.202	45.34	3.1	13.692	B
	2	86.00	450.53	65.50	0.00	184.44	0.466	83.55	9.3	37.016	D

08:30 - 08:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	112.00	462.53	48.50	0.00	140.21	0.799	105.48	15.6	64.306	E
	2	66.00	358.65	35.50	0.00	79.58	0.829	61.87	11.1	83.595	F
B	1	115.00	437.65	60.50	0.00	165.49	0.695	115.67	13.7	49.547	D
C	1	80.00	466.25	50.50	0.00	147.16	0.544	76.84	10.1	49.491	D
	2	60.00	405.43	37.50	0.00	95.02	0.631	70.44	8.8	72.671	E
D	1	99.00	358.65	99.50	0.00	223.04	0.444	95.19	6.9	17.710	B
	2	76.00	455.46	65.50	0.00	186.46	0.408	77.12	8.1	35.505	D

08:45 - 09:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	95.00	466.25	48.50	0.00	141.33	0.672	97.99	12.6	57.140	E

	2	52.00	358.65	35.50	0.00	79.58	0.653	55.15	8.0	70.979	E
B	1	122.00	442.82	60.50	0.00	167.44	0.729	121.07	14.6	51.136	D
C	1	62.00	466.25	50.50	0.00	147.16	0.421	64.39	7.7	45.931	D
	2	55.00	405.43	37.50	0.00	95.02	0.579	55.87	8.0	62.348	E
D	1	51.00	358.65	99.50	0.00	223.04	0.229	54.40	3.5	14.056	B
	2	35.00	443.44	65.50	0.00	181.54	0.193	39.45	3.7	31.013	C

2023, PM WOD

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		49.08	D

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2023	PM WOD	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

		To				
		A	B	C	D	
16:00 - 16:15	From	A	0.00	0.00	79.00	57.00
		B	0.00	0.00	55.00	69.00
		C	67.00	39.00	0.00	0.00
		D	46.00	26.00	2.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:15 - 16:30	From	A	0.00	4.00	68.00	67.00
		B	0.00	0.00	35.00	29.00
		C	77.00	40.00	0.00	0.00
		D	59.00	18.00	4.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:30 - 16:45	From	A	0.00	0.00	68.00	50.00
		B	1.00	0.00	31.00	39.00
		C	63.00	52.00	0.00	0.00
		D	44.00	31.00	0.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:45 - 17:00	From	A	0.00	3.00	69.00	48.00
		B	4.00	0.00	45.00	50.00
		C	73.00	42.00	0.00	0.00
		D	70.00	41.00	5.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:00-16:15	A	136.00	136.00
	B	124.00	124.00
	C	106.00	106.00

	D	74.00	74.00
16:15-16:30	A	139.00	139.00
	B	64.00	64.00
	C	117.00	117.00
	D	81.00	81.00
16:30-16:45	A	118.00	118.00
	B	71.00	71.00
	C	115.00	115.00
	D	75.00	75.00
16:45-17:00	A	120.00	120.00
	B	99.00	99.00
	C	115.00	115.00
	D	116.00	116.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	0.64	48.74	17.3	D
B	0.84	65.30	17.2	E
C	0.59	56.29	15.9	E
D	0.30	23.38	9.9	C

Main Results for each time segment

16:00 - 16:15

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	79.00	466.25	59.50	0.00	173.39	0.456	69.95	9.0	40.567	D
	2	57.00	358.65	46.50	0.00	104.23	0.547	49.42	7.6	53.854	D
B	1	124.00	448.88	52.50	0.00	147.29	0.842	106.84	17.2	65.297	E
C	1	67.00	466.25	47.50	0.00	138.42	0.484	58.36	8.6	49.763	D
	2	39.00	405.43	34.50	0.00	87.42	0.446	33.35	5.7	59.285	E
D	1	46.00	358.65	102.50	0.00	229.76	0.200	43.03	3.0	12.439	B
	2	28.00	456.47	57.50	0.00	164.04	0.171	24.79	3.2	35.653	D

16:15 - 16:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	72.00	458.61	59.50	0.00	170.54	0.422	72.82	8.2	39.769	D
	2	67.00	358.65	46.50	0.00	104.23	0.643	65.46	9.1	58.372	E
B	1	64.00	442.45	52.50	0.00	145.18	0.441	73.31	7.9	45.598	D
C	1	77.00	466.25	47.50	0.00	138.42	0.556	75.61	10.0	52.165	D
	2	40.00	405.43	34.50	0.00	87.42	0.458	39.84	5.8	59.789	E
D	1	59.00	358.65	102.50	0.00	229.76	0.257	58.15	3.8	13.176	B
	2	22.00	442.13	57.50	0.00	158.89	0.138	22.69	2.5	35.099	D

16:30 - 16:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	68.00	466.25	59.50	0.00	173.39	0.392	68.48	7.7	38.972	D
	2	50.00	358.65	46.50	0.00	104.23	0.480	52.54	6.6	51.712	D
B	1	71.00	448.43	52.50	0.00	147.14	0.483	70.11	8.7	46.297	D
C	1	63.00	466.25	47.50	0.00	138.42	0.455	64.93	8.1	49.049	D
	2	52.00	405.43	34.50	0.00	87.42	0.595	50.05	7.8	65.072	E
D	1	44.00	358.65	102.50	0.00	229.76	0.192	44.98	2.8	12.335	B
	2	31.00	466.25	57.50	0.00	167.56	0.185	29.96	3.6	35.900	D

16:45 - 17:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	72.00	460.49	59.50	0.00	171.25	0.420	71.52	8.2	39.690	D
	2	48.00	358.65	46.50	0.00	104.23	0.461	48.28	6.3	50.935	D
B	1	99.00	445.66	52.50	0.00	146.23	0.677	95.10	12.6	53.731	D
C	1	73.00	466.25	47.50	0.00	138.42	0.527	71.63	9.5	51.171	D
	2	42.00	405.43	34.50	0.00	87.42	0.480	43.63	6.1	60.815	E
D	1	70.00	358.65	102.50	0.00	229.76	0.305	68.29	4.6	13.863	B
	2	46.00	451.53	57.50	0.00	162.27	0.283	44.25	5.3	37.865	D

<h1>Junctions 9</h1>
<h2>OSCADY 9 - Signalised Intersection Module</h2>
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Filename: Sutton Cross 2023 WITH COMMITTED.j9
Path: C:\Users\Martin.Rogers\Documents\howth road
Report generation date: 13/04/2020 17:16:20

»2023, AM with committed
 »2023, PM with committed

Summary of junction performance

	AM with committed				PM with committed			
	Queue (PCU)	Delay (s)	DOS	LOS	Queue (PCU)	Delay (s)	DOS	LOS
2023								
Arm A	30.8	74.20	0.87	E	19.4	51.93	0.71	D
Arm B	16.3	57.93	0.80	E	19.4	72.01	0.90	E
Arm C	29.3	88.25	1.04	F	18.9	57.79	0.67	E
Arm D	15.7	30.06	0.50	C	11.0	23.86	0.35	C

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle.

File summary

File Description

Title	Sutton Cross
Location	Howth
Site number	
Date	10/04/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Min	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	DOS Threshold	Average Delay threshold (s)	Queue threshold (PCU)
		0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM with committed	DIRECT	08:00	09:00	60	15
D2	2023	PM with committed	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2023, AM with committed

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		63.23	E

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description
A	Howth Road	
B	Carrickrock Road	
C	Dublin Road	
D	Station Road	

OSCADY Traffic Streams

Arm	Traffic Stream	Phase	Notional EEG (s)	Signals EEG (s)	Destination arms	Straight move
A	1	E	0.00	0.00	B, C	C
	2	D	0.00	0.00	D	C
B	1	F	0.00	0.00	A, C, D	D
C	1	B	0.00	1.00	A	A
	2	A	0.00	0.00	B	A
D	1	G	0.00	0.00	A	B
	2	C	0.00	5.00	B, C	B

OSCADY Lanes

Arm	Traffic Stream	Destination arms	Gradient (%)	Width (m)	Turning radius (m)	Nearside lane	Has bay
A	1	B, C	0	2.50	5.00	✓	
	2	D	0	2.50	5.00	✓	
B	1	A, C, D	0	3.00	10.00	✓	
C	1	A	0	2.50		✓	
	2	B	0	2.50	10.00	✓	
D	1	A	0	2.50	5.00	✓	
	2	B, C	0	2.50	5.00	✓	

Signal Timings**Junction 1**

Junction	Sequence to use	Cycle time (s)	Maximum cycle time (s)	Start displacement (s)	End displacement (s)
1	1	160	300	1.40	2.90

Optimisation options

Junction	Optimise stage lengths	Optimise cycle time	Optimiser demand source	Optimiser message
1	✓		Average	Timings provide delay minimisation.

Phases

Junction	Phase	Name	Minimum green (s)
1	A		15
	B		15
	C		15
	D		15
	E		15
	F		15
	G		15

Library Stages

Junction	Library Stage	Phases in stage	User stage minimum (s)	Run every N cycles	Probability of running (%)
1	1	B, E	7		
	2	D, E, G	7		
	3	C, G, F	7		
	4		7		
	5	A, B	7		

Losing / Gaining Phase Delays

Junction	Delay	Type	Phase	From stage	To stage	Relative delay (s)
1	1	Losing	B	1	2	1

Stage Sequences

Junction	Sequence	Name	Stage IDs	Stage ends
1	1		1, 2, 3, 4, 5	12, 55, 117, 125, 0

Intergreen Matrix for Junction 1

		To						
		A	B	C	D	E	F	G
From	A			5	5	5	5	5
	B			5	5		5	5
	C	0	0		0	5		

	D	0	0	0			5	
	E	0		0			0	
	F	0	0		0	5		
	G	0	0					

Interstage Matrix for Junction 1

		To				
		1	2	3	4	5
From	1	0	6	5	0	0
	2	0	0	5	0	0
	3	5	5	0	0	0
	4	0	0	0	0	0
	5	5	5	5	0	0

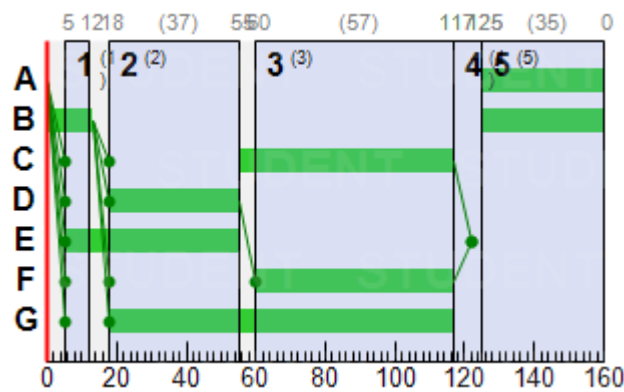
Resultant Stages

Junction	Resultant Stage	Library Stage ID	Phases in this stage	Stage start (s)	Stage end (s)	Stage duration (s)	User stage minimum (s)	Stage minimum (s)
1	1	1	B,E	5	12	7	7	7
	2	2	D,E,G	18	55	37	7	15
	3	3	C,G,F	60	117	57	7	15
	4	4		117	125	8	7	7
	5	5	A,B	125	0	35	7	15

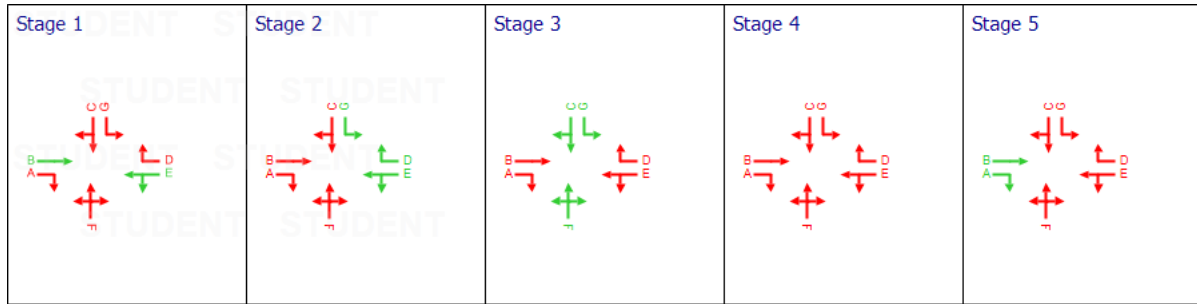
Resultant Phase Green Periods

Junction	Phase	Green period	Start time (s)	End time (s)	Duration (s)
1	A	1	125	0	35
	B	1	125	13	48
	C	1	55	117	62
	D	1	18	55	37
	E	1	5	55	50
	F	1	60	117	57
	G	1	18	117	99

Phase Timings Diagram for Junction 1



Stage Sequence Diagram for Junction 1



Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM with committed	DIRECT	08:00	09:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:00 - 08:15

		To			
		A	B	C	D
From	A	0.00	0.00	74.00	47.00
	B	0.00	0.00	55.00	64.00
	C	55.00	52.00	0.00	0.00
	D	54.00	64.00	4.00	0.00

Demand (PCU/TS)

08:15 - 08:30

		To			
		A	B	C	D
From	A	0.00	0.00	88.00	55.00
	B	2.00	0.00	76.00	49.00
	C	60.00	96.00	0.00	0.00
	D	49.00	79.00	10.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	D
From	A	0.00	3.00	126.00	74.00
	B	2.00	0.00	74.00	46.00
	C	85.00	63.00	0.00	0.00
	D	103.00	73.00	6.00	0.00

08:30 - 08:45

Demand (PCU/TS)

		To			
		A	B	C	D
From	A	0.00	0.00	112.00	60.00
	B	1.00	0.00	69.00	59.00
	C	66.00	57.00	0.00	0.00
	D	55.00	32.00	6.00	0.00

08:45 - 09:00

Vehicle Mix

Heavy Vehicle Percentages

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:00-08:15	A	121.00	121.00
	B	119.00	119.00
	C	107.00	107.00
	D	122.00	122.00
08:15-08:30	A	143.00	143.00
	B	127.00	127.00
	C	156.00	156.00
	D	138.00	138.00
08:30-08:45	A	203.00	203.00
	B	122.00	122.00
	C	148.00	148.00
	D	182.00	182.00
08:45-09:00	A	172.00	172.00
	B	129.00	129.00
	C	123.00	123.00
	D	93.00	93.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	0.87	74.20	30.8	E
B	0.80	57.93	16.3	E
C	1.04	88.25	29.3	F
D	0.50	30.06	15.7	C

Main Results for each time segment**08:00 - 08:15**

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	74.00	466.25	51.50	0.00	150.07	0.493	64.79	9.2	47.150	D
	2	47.00	358.65	38.50	0.00	86.30	0.545	40.27	6.7	60.203	E
B	1	119.00	447.71	58.50	0.00	163.69	0.727	104.47	14.5	51.933	D
C	1	55.00	466.25	49.50	0.00	144.25	0.381	48.11	6.9	45.537	D
	2	52.00	405.43	36.50	0.00	92.49	0.562	44.44	7.6	61.794	E
D	1	54.00	358.65	100.50	0.00	225.28	0.240	50.38	3.6	13.777	B
	2	68.00	458.16	63.50	0.00	181.83	0.374	60.58	7.4	35.929	D

08:15 - 08:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	88.00	466.25	51.50	0.00	150.07	0.586	86.11	11.1	50.319	D
	2	55.00	358.65	38.50	0.00	86.30	0.637	53.66	8.1	64.878	E
B	1	127.00	438.37	58.50	0.00	160.28	0.792	125.52	16.0	57.111	E
C	1	60.00	466.25	49.50	0.00	144.25	0.416	59.35	7.5	46.441	D
	2	96.00	405.43	36.50	0.00	92.49	1.038	81.83	21.7	114.380	F
D	1	49.00	358.65	100.50	0.00	225.28	0.218	49.34	3.3	13.481	B
	2	89.00	451.05	63.50	0.00	179.01	0.497	86.59	9.8	39.192	D

08:30 - 08:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	129.00	463.02	51.50	0.00	149.03	0.866	121.74	18.4	68.509	E
	2	74.00	358.65	38.50	0.00	86.30	0.857	69.64	12.4	84.126	F
B	1	122.00	437.84	58.50	0.00	160.08	0.762	122.73	15.3	55.769	E
C	1	85.00	466.25	49.50	0.00	144.25	0.589	81.61	10.9	51.856	D
	2	63.00	405.43	36.50	0.00	92.49	0.681	74.03	10.7	95.494	F
D	1	103.00	358.65	100.50	0.00	225.28	0.457	99.24	7.0	17.516	B
	2	79.00	455.86	63.50	0.00	180.92	0.437	80.16	8.7	37.537	D

08:45 - 09:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	112.00	466.25	51.50	0.00	150.07	0.746	115.47	14.9	60.096	E

	2	60.00	358.65	38.50	0.00	86.30	0.695	63.31	9.1	71.644	E
B	1	129.00	442.72	58.50	0.00	161.87	0.797	127.97	16.3	57.930	E
C	1	66.00	466.25	49.50	0.00	144.25	0.458	68.60	8.3	47.669	D
	2	57.00	405.43	36.50	0.00	92.49	0.616	59.27	8.4	65.349	E
D	1	55.00	358.65	100.50	0.00	225.28	0.244	58.36	3.7	13.846	B
	2	38.00	445.16	63.50	0.00	176.67	0.215	42.57	4.1	32.660	C

2023, PM with committed

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		51.81	D

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2023	PM with committed	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

		To				
		A	B	C	D	
16:00 - 16:15	From	A	0.00	0.00	88.00	62.00
		B	0.00	0.00	60.00	72.00
		C	84.00	44.00	0.00	0.00
		D	56.00	29.00	2.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:15 - 16:30	From	A	0.00	4.00	76.00	72.00
		B	0.00	0.00	40.00	32.00
		C	94.00	45.00	0.00	0.00
		D	69.00	21.00	4.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:30 - 16:45	From	A	0.00	0.00	77.00	56.00
		B	1.00	0.00	36.00	42.00
		C	79.00	57.00	0.00	0.00
		D	54.00	35.00	0.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:45 - 17:00	From	A	0.00	3.00	77.00	53.00
		B	4.00	0.00	50.00	53.00
		C	89.00	46.00	0.00	0.00
		D	80.00	44.00	5.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:00-16:15	A	150.00	150.00
	B	132.00	132.00
	C	128.00	128.00

	D	87.00	87.00
16:15-16:30	A	152.00	152.00
	B	72.00	72.00
	C	139.00	139.00
	D	94.00	94.00
16:30-16:45	A	133.00	133.00
	B	79.00	79.00
	C	136.00	136.00
	D	89.00	89.00
16:45-17:00	A	133.00	133.00
	B	107.00	107.00
	C	135.00	135.00
	D	129.00	129.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	0.71	51.93	19.4	D
B	0.90	72.01	19.4	E
C	0.67	57.79	18.9	E
D	0.35	23.86	11.0	C

Main Results for each time segment

16:00 - 16:15

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	88.00	466.25	58.50	0.00	170.47	0.516	77.75	10.3	42.989	D
	2	62.00	358.65	45.50	0.00	101.99	0.608	53.56	8.4	57.255	E
B	1	132.00	448.19	52.50	0.00	147.06	0.898	112.63	19.4	72.013	E
C	1	84.00	466.25	48.50	0.00	141.33	0.594	73.08	10.9	52.760	D
	2	44.00	405.43	35.50	0.00	89.96	0.489	37.64	6.4	59.876	E
D	1	56.00	358.65	101.50	0.00	227.52	0.246	52.31	3.7	13.445	B
	2	31.00	457.40	57.50	0.00	164.38	0.189	27.44	3.6	35.980	D

16:15 - 16:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	80.00	459.36	58.50	0.00	167.95	0.476	80.97	9.3	41.934	D
	2	72.00	358.65	45.50	0.00	101.99	0.706	70.31	10.1	63.029	E
B	1	72.00	441.92	52.50	0.00	145.01	0.497	82.47	8.9	47.831	D
C	1	94.00	466.25	48.50	0.00	141.33	0.665	92.50	12.4	55.968	E
	2	45.00	405.43	35.50	0.00	89.96	0.500	44.84	6.5	60.456	E
D	1	69.00	358.65	101.50	0.00	227.52	0.303	68.12	4.6	14.272	B
	2	25.00	444.90	57.50	0.00	159.88	0.156	25.69	2.9	35.414	D

16:30 - 16:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	77.00	466.25	58.50	0.00	170.47	0.452	77.38	8.9	41.188	D
	2	56.00	358.65	45.50	0.00	101.99	0.549	58.59	7.5	55.302	E
B	1	79.00	447.32	52.50	0.00	146.78	0.538	78.09	9.8	48.096	D
C	1	79.00	466.25	48.50	0.00	141.33	0.559	81.19	10.2	51.771	D
	2	57.00	405.43	35.50	0.00	89.96	0.634	55.00	8.5	66.122	E
D	1	54.00	358.65	101.50	0.00	227.52	0.237	55.01	3.6	13.331	B
	2	35.00	466.25	57.50	0.00	167.56	0.209	33.85	4.0	36.345	D

16:45 - 17:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	80.00	461.06	58.50	0.00	168.58	0.475	79.63	9.3	41.832	D
	2	53.00	358.65	45.50	0.00	101.99	0.520	53.45	7.1	53.890	D
B	1	107.00	445.06	52.50	0.00	146.03	0.733	102.88	13.9	56.855	E
C	1	89.00	466.25	48.50	0.00	141.33	0.630	87.57	11.7	54.329	D
	2	46.00	405.43	35.50	0.00	89.96	0.511	47.83	6.7	61.169	E
D	1	80.00	358.65	101.50	0.00	227.52	0.352	78.24	5.3	15.044	B
	2	49.00	452.40	57.50	0.00	162.58	0.301	47.36	5.7	38.244	D

<h1>Junctions 9</h1>
<h2>OSCADY 9 - Signalised Intersection Module</h2>
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Filename: Sutton Cross 2023 WITH COMMITTED PLUS DEV.j9

Path: C:\Users\Martin.Rogers\Documents\howth road

Report generation date: 10/05/2020 15:02:52

»2023, AM with committed and dev

»2023, PM with committed and dev

Summary of junction performance

	AM with committed and dev				PM with committed and dev			
	Queue (PCU)	Delay (s)	DOS	LOS	Queue (PCU)	Delay (s)	DOS	LOS
2023								
Arm A	32.0	76.42	0.89	E	20.0	53.82	0.73	D
Arm B	16.5	58.58	0.80	E	20.4	76.61	0.92	E
Arm C	30.1	89.35	1.05	F	18.9	55.59	0.65	E
Arm D	15.8	29.96	0.50	C	11.4	24.76	0.37	C

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle.

File summary

File Description

Title	Sutton Cross
Location	Howth
Site number	
Date	10/04/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Min	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	DOS Threshold	Average Delay threshold (s)	Queue threshold (PCU)
		0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM with committed and dev	DIRECT	08:00	09:00	60	15
D2	2023	PM with committed and dev	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2023, AM with committed and dev

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		64.33	E

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description
A	Howth Road	
B	Carrickrock Road	
C	Dublin Road	
D	Station Road	

OSCADY Traffic Streams

Arm	Traffic Stream	Phase	Notional EEG (s)	Signals EEG (s)	Destination arms	Straight move
A	1	E	0.00	0.00	B, C	C
	2	D	0.00	0.00	D	C
B	1	F	0.00	0.00	A, C, D	D
C	1	B	0.00	1.00	A	A
	2	A	0.00	0.00	B	A
D	1	G	0.00	0.00	A	B
	2	C	0.00	5.00	B, C	B

OSCADY Lanes

Arm	Traffic Stream	Destination arms	Gradient (%)	Width (m)	Turning radius (m)	Nearside lane	Has bay
A	1	B, C	0	2.50	5.00	✓	
	2	D	0	2.50	5.00	✓	
B	1	A, C, D	0	3.00	10.00	✓	
C	1	A	0	2.50		✓	
	2	B	0	2.50	10.00	✓	
D	1	A	0	2.50	5.00	✓	
	2	B, C	0	2.50	5.00	✓	

Signal Timings**Junction 1**

Junction	Sequence to use	Cycle time (s)	Maximum cycle time (s)	Start displacement (s)	End displacement (s)
1	1	160	300	1.40	2.90

Optimisation options

Junction	Optimise stage lengths	Optimise cycle time	Optimiser demand source	Optimiser message
1	✓		Average	Timings provide delay minimisation.

Phases

Junction	Phase	Name	Minimum green (s)
1	A		15
	B		15
	C		15
	D		15
	E		15
	F		15
	G		15

Library Stages

Junction	Library Stage	Phases in stage	User stage minimum (s)	Run every N cycles	Probability of running (%)
1	1	B, E	7		
	2	D, E, G	7		
	3	C, G, F	7		
	4		7		
	5	A, B	7		

Losing / Gaining Phase Delays

Junction	Delay	Type	Phase	From stage	To stage	Relative delay (s)
1	1	Losing	B	1	2	1

Stage Sequences

Junction	Sequence	Name	Stage IDs	Stage ends
1	1		1, 2, 3, 4, 5	12, 55, 117, 125, 0

Intergreen Matrix for Junction 1

		To						
		A	B	C	D	E	F	G
From	A			5	5	5	5	5
	B			5	5		5	5
	C	0	0		0	5		

	D	0	0	0			5	
	E	0		0			0	
	F	0	0		0	5		
	G	0	0					

Interstage Matrix for Junction 1

		To				
		1	2	3	4	5
From	1	0	6	5	0	0
	2	0	0	5	0	0
	3	5	5	0	0	0
	4	0	0	0	0	0
	5	5	5	5	0	0

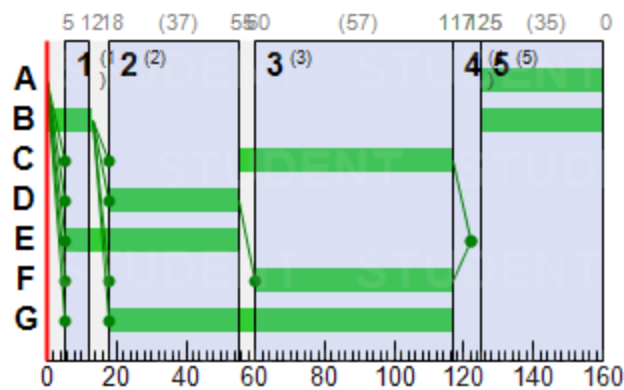
Resultant Stages

Junction	Resultant Stage	Library Stage ID	Phases in this stage	Stage start (s)	Stage end (s)	Stage duration (s)	User stage minimum (s)	Stage minimum (s)
1	1	1	B,E	5	12	7	7	7
	2	2	D,E,G	18	55	37	7	15
	3	3	C,G,F	60	117	57	7	15
	4	4		117	125	8	7	7
	5	5	A,B	125	0	35	7	15

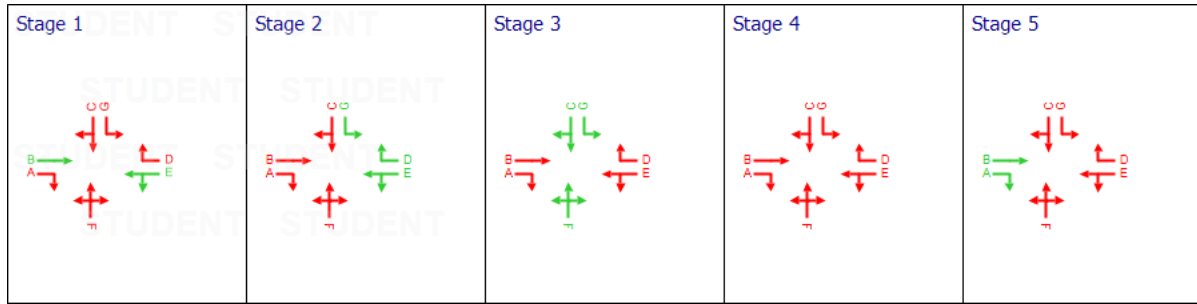
Resultant Phase Green Periods

Junction	Phase	Green period	Start time (s)	End time (s)	Duration (s)
1	A	1	125	0	35
	B	1	125	13	48
	C	1	55	117	62
	D	1	18	55	37
	E	1	5	55	50
	F	1	60	117	57
	G	1	18	117	99

Phase Timings Diagram for Junction 1



Stage Sequence Diagram for Junction 1



Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM with committed and dev	DIRECT	08:00	09:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:00 - 08:15

		To			
		A	B	C	D
From	A	0.00	0.00	77.00	48.00
	B	0.00	0.00	56.00	65.00
	C	56.00	52.00	0.00	0.00
	D	55.00	64.00	4.00	0.00

Demand (PCU/TS)

08:15 - 08:30

		To			
		A	B	C	D
From	A	0.00	0.00	91.00	57.00
	B	2.00	0.00	77.00	49.00
	C	61.00	97.00	0.00	0.00
	D	50.00	79.00	10.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	D
08:30 - 08:45	From A	0.00	3.00	129.00	75.00
	B	2.00	0.00	75.00	46.00
	C	85.00	63.00	0.00	0.00
	D	104.00	73.00	6.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	D
08:45 - 09:00	From A	0.00	0.00	116.00	62.00
	B	1.00	0.00	70.00	59.00
	C	67.00	57.00	0.00	0.00
	D	56.00	32.00	6.00	0.00

Vehicle Mix**Heavy Vehicle Percentages**

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data**Demand for each time segment**

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:00-08:15	A	125.00	125.00
	B	121.00	121.00
	C	108.00	108.00
	D	123.00	123.00
08:15-08:30	A	148.00	148.00
	B	128.00	128.00
	C	158.00	158.00
	D	139.00	139.00
08:30-08:45	A	207.00	207.00
	B	123.00	123.00
	C	148.00	148.00
	D	183.00	183.00
08:45-09:00	A	178.00	178.00
	B	130.00	130.00
	C	124.00	124.00
	D	94.00	94.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	0.89	76.42	32.0	E
B	0.80	58.58	16.5	E
C	1.05	89.35	30.1	F
D	0.50	29.96	15.8	C

Main Results for each time segment**08:00 - 08:15**

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	77.00	466.25	51.50	0.00	150.07	0.513	67.40	9.6	47.763	D
	2	48.00	358.65	38.50	0.00	86.30	0.556	41.11	6.9	60.691	E
B	1	121.00	447.67	58.50	0.00	163.68	0.739	106.15	14.8	52.661	D
C	1	56.00	466.25	49.50	0.00	144.25	0.388	48.98	7.0	45.709	D
	2	52.00	405.43	36.50	0.00	92.49	0.562	44.44	7.6	61.794	E
D	1	55.00	358.65	100.50	0.00	225.28	0.244	51.32	3.7	13.838	B
	2	68.00	458.16	63.50	0.00	181.83	0.374	60.58	7.4	35.929	D

08:15 - 08:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	91.00	466.25	51.50	0.00	150.07	0.606	89.08	11.5	51.096	D
	2	57.00	358.65	38.50	0.00	86.30	0.660	55.46	8.4	66.218	E
B	1	128.00	438.18	58.50	0.00	160.21	0.799	126.64	16.2	57.735	E
C	1	61.00	466.25	49.50	0.00	144.25	0.423	60.35	7.7	46.625	D
	2	97.00	405.43	36.50	0.00	92.49	1.049	82.16	22.4	116.216	F
D	1	50.00	358.65	100.50	0.00	225.28	0.222	50.34	3.3	13.540	B
	2	89.00	451.05	63.50	0.00	179.01	0.497	86.59	9.8	39.192	D

08:30 - 08:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	132.00	463.09	51.50	0.00	149.06	0.886	124.32	19.2	71.025	E
	2	75.00	358.65	38.50	0.00	86.30	0.869	70.67	12.8	85.905	F
B	1	123.00	437.65	58.50	0.00	160.02	0.769	123.73	15.5	56.352	E
C	1	85.00	466.25	49.50	0.00	144.25	0.589	81.74	10.9	51.858	D
	2	63.00	405.43	36.50	0.00	92.49	0.681	74.60	10.8	99.015	F
D	1	104.00	358.65	100.50	0.00	225.28	0.462	100.23	7.1	17.612	B
	2	79.00	455.86	63.50	0.00	180.92	0.437	80.16	8.7	37.537	D

08:45 - 09:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	116.00	466.25	51.50	0.00	150.07	0.773	119.53	15.7	62.730	E

	2	62.00	358.65	38.50	0.00	86.30	0.718	65.21	9.6	74.142	E
B	1	130.00	442.50	58.50	0.00	161.79	0.804	128.96	16.5	58.581	E
C	1	67.00	466.25	49.50	0.00	144.25	0.464	69.46	8.5	47.873	D
	2	57.00	405.43	36.50	0.00	92.49	0.616	59.36	8.4	65.351	E
D	1	56.00	358.65	100.50	0.00	225.28	0.249	59.36	3.8	13.908	B
	2	38.00	445.16	63.50	0.00	176.67	0.215	42.57	4.1	32.660	C

2023, PM with committed and dev

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		52.82	D

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2023	PM with committed and dev	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

		To				
		A	B	C	D	
16:00 - 16:15	From	A	0.00	0.00	88.00	63.00
	B	0.00	0.00	61.00	72.00	
	C	86.00	45.00	0.00	0.00	
	D	58.00	30.00	2.00	0.00	

Demand (PCU/TS)

		To				
		A	B	C	D	
16:15 - 16:30	From	A	0.00	4.00	77.00	73.00
	B	0.00	0.00	41.00	32.00	
	C	96.00	46.00	0.00	0.00	
	D	71.00	21.00	4.00	0.00	

Demand (PCU/TS)

		To				
		A	B	C	D	
16:30 - 16:45	From	A	0.00	0.00	78.00	56.00
	B	1.00	0.00	37.00	42.00	
	C	82.00	58.00	0.00	0.00	
	D	56.00	35.00	0.00	0.00	

Demand (PCU/TS)

		To				
		A	B	C	D	
16:45 - 17:00	From	A	0.00	3.00	78.00	54.00
	B	4.00	0.00	50.00	53.00	
	C	92.00	47.00	0.00	0.00	
	D	82.00	44.00	5.00	0.00	

Vehicle Mix

Heavy Vehicle Percentages

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:00-16:15	A	151.00	151.00
	B	133.00	133.00
	C	131.00	131.00

	D	90.00	90.00
16:15-16:30	A	154.00	154.00
	B	73.00	73.00
	C	142.00	142.00
	D	96.00	96.00
16:30-16:45	A	134.00	134.00
	B	80.00	80.00
	C	140.00	140.00
	D	91.00	91.00
16:45-17:00	A	135.00	135.00
	B	107.00	107.00
	C	139.00	139.00
	D	131.00	131.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	0.73	53.82	20.0	D
B	0.92	76.61	20.4	E
C	0.65	55.59	18.9	E
D	0.37	24.76	11.4	C

Main Results for each time segment

16:00 - 16:15

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	88.00	466.25	57.50	0.00	167.56	0.525	77.63	10.4	43.950	D
	2	63.00	358.65	44.50	0.00	99.75	0.632	54.29	8.7	59.214	E
B	1	133.00	447.93	51.50	0.00	144.18	0.922	112.55	20.4	76.611	E
C	1	86.00	466.25	50.50	0.00	147.16	0.584	75.05	10.9	50.913	D
	2	45.00	405.43	37.50	0.00	95.02	0.474	38.62	6.4	57.700	E
D	1	58.00	358.65	99.50	0.00	223.04	0.260	54.05	4.0	14.492	B
	2	32.00	457.67	56.50	0.00	161.61	0.198	28.29	3.7	36.814	D

16:15 - 16:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	81.00	459.44	57.50	0.00	165.11	0.491	81.86	9.5	43.036	D
	2	73.00	358.65	44.50	0.00	99.75	0.732	71.22	10.5	65.780	E
B	1	73.00	441.55	51.50	0.00	142.12	0.514	84.32	9.1	49.673	D
C	1	96.00	466.25	50.50	0.00	147.16	0.652	94.55	12.4	53.847	D
	2	46.00	405.43	37.50	0.00	95.02	0.484	45.85	6.5	58.200	E
D	1	71.00	358.65	99.50	0.00	223.04	0.318	70.09	4.9	15.388	B
	2	25.00	444.90	56.50	0.00	157.10	0.159	25.82	2.9	36.121	D

16:30 - 16:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	78.00	466.25	57.50	0.00	167.56	0.466	78.38	9.1	42.253	D
	2	56.00	358.65	44.50	0.00	99.75	0.561	58.86	7.6	56.710	E
B	1	80.00	446.91	51.50	0.00	143.85	0.556	79.07	10.1	49.450	D
C	1	82.00	466.25	50.50	0.00	147.16	0.557	83.99	10.4	50.214	D
	2	58.00	405.43	37.50	0.00	95.02	0.610	56.08	8.4	63.186	E
D	1	56.00	358.65	99.50	0.00	223.04	0.251	57.05	3.8	14.369	B
	2	35.00	466.25	56.50	0.00	164.64	0.213	33.83	4.1	37.075	D

16:45 - 17:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	81.00	461.13	57.50	0.00	165.72	0.489	80.63	9.5	42.927	D
	2	54.00	358.65	44.50	0.00	99.75	0.541	54.31	7.3	55.551	E
B	1	107.00	445.06	51.50	0.00	143.25	0.747	102.91	14.1	58.544	E
C	1	92.00	466.25	50.50	0.00	147.16	0.625	90.60	11.8	52.640	D
	2	47.00	405.43	37.50	0.00	95.02	0.495	48.76	6.7	58.795	E
D	1	82.00	358.65	99.50	0.00	223.04	0.368	80.18	5.6	16.227	B
	2	49.00	452.40	56.50	0.00	159.75	0.307	47.34	5.7	39.028	D

Junctions 9

OSCADY 9 - Signalised Intersection Module

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The users of this computer program for the solution of an engineering problem are in no way relieved of their responsibility for the correctness of the solution

Filename: Sutton Cross 2038 WOD.j9

Path: C:\Users\Martin.Rogers\Documents\howth road

Report generation date: 10/05/2020 15:24:25

»2038, AM WOD

»2038, PM WOD

Summary of junction performance

	AM WOD				PM WOD			
	Queue (PCU)	Delay (s)	DOS	LOS	Queue (PCU)	Delay (s)	DOS	LOS
2038								
Arm A	36.0	88.96	0.96	F	20.7	54.17	0.75	D
Arm B	18.5	63.87	0.86	E	23.2	81.49	0.96	F
Arm C	36.4	96.72	1.12	F	18.6	59.74	0.69	E
Arm D	17.7	31.45	0.55	C	11.4	23.88	0.35	C

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle.

File summary

File Description

Title	Sutton Cross
Location	Howth
Site number	
Date	10/04/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Min	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	DOS Threshold	Average Delay threshold (s)	Queue threshold (PCU)
		0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2038	AM WOD	DIRECT	08:00	09:00	60	15
D2	2038	PM WOD	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2038, AM WOD

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		70.74	E

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description
A	Howth Road	
B	Carrickrock Road	
C	Dublin Road	
D	Station Road	

OSCADY Traffic Streams

Arm	Traffic Stream	Phase	Notional EEG (s)	Signals EEG (s)	Destination arms	Straight move
A	1	E	0.00	0.00	B, C	C
	2	D	0.00	0.00	D	C
B	1	F	0.00	0.00	A, C, D	D
C	1	B	0.00	1.00	A	A
	2	A	0.00	0.00	B	A
D	1	G	0.00	0.00	A	B
	2	C	0.00	5.00	B, C	B

OSCADY Lanes

Arm	Traffic Stream	Destination arms	Gradient (%)	Width (m)	Turning radius (m)	Nearside lane	Has bay
A	1	B, C	0	2.50	5.00	✓	
	2	D	0	2.50	5.00	✓	
B	1	A, C, D	0	3.00	10.00	✓	
C	1	A	0	2.50		✓	
	2	B	0	2.50	10.00	✓	
D	1	A	0	2.50	5.00	✓	
	2	B, C	0	2.50	5.00	✓	

Signal Timings**Junction 1**

Junction	Sequence to use	Cycle time (s)	Maximum cycle time (s)	Start displacement (s)	End displacement (s)
1	1	160	300	1.40	2.90

Optimisation options

Junction	Optimise stage lengths	Optimise cycle time	Optimiser demand source	Optimiser message
1	✓		Average	Timings provide delay minimisation.

Phases

Junction	Phase	Name	Minimum green (s)
1	A		15
	B		15
	C		15
	D		15
	E		15
	F		15
	G		15

Library Stages

Junction	Library Stage	Phases in stage	User stage minimum (s)	Run every N cycles	Probability of running (%)
1	1	B, E	7		
	2	D, E, G	7		
	3	C, G, F	7		
	4		7		
	5	A, B	7		

Losing / Gaining Phase Delays

Junction	Delay	Type	Phase	From stage	To stage	Relative delay (s)
1	1	Losing	B	1	2	1

Stage Sequences

Junction	Sequence	Name	Stage IDs	Stage ends
1	1		1, 2, 3, 4, 5	12, 52, 115, 123, 0

Intergreen Matrix for Junction 1

		To						
		A	B	C	D	E	F	G
From	A			5	5	5	5	5
	B			5	5		5	5
	C	0	0		0	5		

	D	0	0	0			5	
	E	0		0			0	
	F	0	0		0	5		
	G	0	0					

Interstage Matrix for Junction 1

From	To					
		1	2	3	4	5
	1	0	6	5	0	0
	2	0	0	5	0	0
	3	5	5	0	0	0
	4	0	0	0	0	0
5	5	5	5	0	0	

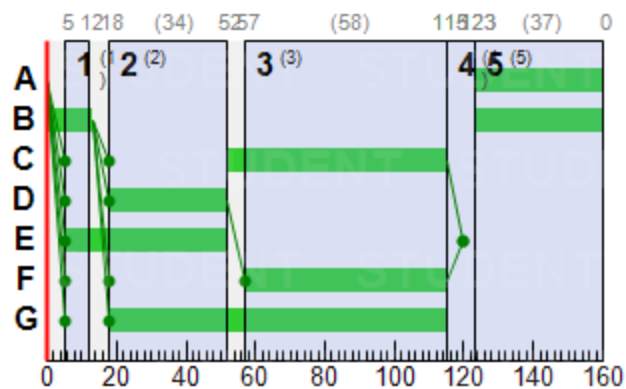
Resultant Stages

Junction	Resultant Stage	Library Stage ID	Phases in this stage	Stage start (s)	Stage end (s)	Stage duration (s)	User stage minimum (s)	Stage minimum (s)
1	1	1	B,E	5	12	7	7	7
	2	2	D,E,G	18	52	34	7	15
	3	3	C,G,F	57	115	58	7	15
	4	4		115	123	8	7	7
	5	5	A,B	123	0	37	7	15

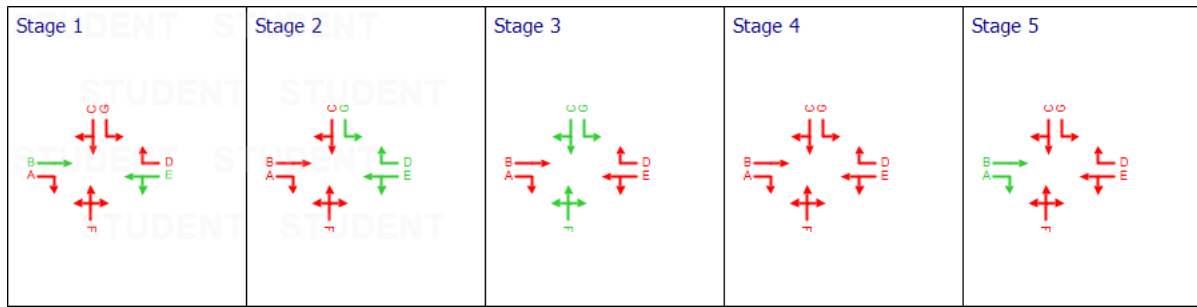
Resultant Phase Green Periods

Junction	Phase	Green period	Start time (s)	End time (s)	Duration (s)
1	A	1	123	0	37
	B	1	123	13	50
	C	1	52	115	63
	D	1	18	52	34
	E	1	5	52	47
	F	1	57	115	58
	G	1	18	115	97

Phase Timings Diagram for Junction 1



Stage Sequence Diagram for Junction 1



Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2038	AM WOD	DIRECT	08:00	09:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:00 - 08:15

		To			
		A	B	C	D
From	A	0.00	6.00	66.00	45.00
	B	0.00	0.00	59.00	71.00
	C	59.00	57.00	0.00	0.00
	D	57.00	70.00	5.00	0.00

Demand (PCU/TS)

08:15 - 08:30

		To			
		A	B	C	D
From	A	0.00	0.00	82.00	55.00
	B	2.00	0.00	83.00	53.00
	C	65.00	109.00	0.00	0.00
	D	52.00	88.00	12.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
08:30 - 08:45	From	A	0.00	4.00	126.00	76.00
		B	2.00	0.00	81.00	49.00
		C	93.00	70.00	0.00	0.00
		D	114.00	81.00	7.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
08:45 - 09:00	From	A	0.00	0.00	110.00	61.00
		B	1.00	0.00	75.00	65.00
		C	71.00	63.00	0.00	0.00
		D	59.00	34.00	7.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:00-08:15	A	117.00	117.00
	B	130.00	130.00
	C	116.00	116.00
	D	132.00	132.00
08:15-08:30	A	137.00	137.00
	B	138.00	138.00
	C	174.00	174.00
	D	152.00	152.00
08:30-08:45	A	206.00	206.00
	B	132.00	132.00
	C	163.00	163.00
	D	202.00	202.00
08:45-09:00	A	171.00	171.00
	B	141.00	141.00
	C	134.00	134.00
	D	100.00	100.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	0.96	88.96	36.0	F
B	0.86	63.87	18.5	E
C	1.12	96.72	36.4	F
D	0.55	31.45	17.7	C

Main Results for each time segment**08:00 - 08:15**

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	72.00	454.88	48.50	0.00	137.88	0.522	62.74	9.3	50.316	D
	2	45.00	358.65	35.50	0.00	79.58	0.565	38.35	6.7	63.697	E
B	1	130.00	448.24	59.50	0.00	166.69	0.780	113.93	16.1	54.612	D
C	1	59.00	466.25	51.50	0.00	150.07	0.393	51.74	7.3	44.418	D
	2	57.00	405.43	38.50	0.00	97.56	0.584	48.83	8.2	61.036	E
D	1	57.00	358.65	98.50	0.00	220.80	0.258	53.05	3.9	14.900	B
	2	75.00	457.11	64.50	0.00	184.27	0.407	66.88	8.1	36.083	D

08:15 - 08:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	82.00	466.25	48.50	0.00	141.33	0.580	80.63	10.6	52.315	D
	2	55.00	358.65	35.50	0.00	79.58	0.691	53.17	8.5	71.019	E
B	1	138.00	438.26	59.50	0.00	162.98	0.847	136.16	17.9	61.803	E
C	1	65.00	466.25	51.50	0.00	150.07	0.433	64.23	8.0	45.476	D
	2	109.00	405.43	38.50	0.00	97.56	1.117	88.84	28.3	127.274	F
D	1	52.00	358.65	98.50	0.00	220.80	0.236	52.35	3.6	14.579	B
	2	100.00	450.05	64.50	0.00	181.43	0.551	97.11	11.0	40.222	D

08:30 - 08:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	130.00	461.99	48.50	0.00	140.04	0.928	119.91	20.7	79.960	E
	2	76.00	358.65	35.50	0.00	79.58	0.955	69.23	15.2	104.364	F
B	1	132.00	437.49	59.50	0.00	162.69	0.811	132.94	17.0	60.218	E
C	1	93.00	466.25	51.50	0.00	150.07	0.620	89.23	11.8	51.591	D
	2	70.00	405.43	38.50	0.00	97.56	0.718	85.75	12.6	131.889	F
D	1	114.00	358.65	98.50	0.00	220.80	0.516	109.48	8.1	19.894	B
	2	88.00	455.38	64.50	0.00	183.58	0.479	89.41	9.6	38.071	D

08:45 - 09:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	110.00	466.25	48.50	0.00	141.33	0.778	115.32	15.4	66.798	E

	2	61.00	358.65	35.50	0.00	79.58	0.767	66.11	10.1	87.917	F
B	1	141.00	442.94	59.50	0.00	164.72	0.856	139.49	18.5	63.869	E
C	1	71.00	466.25	51.50	0.00	150.07	0.473	73.99	8.8	46.697	D
	2	63.00	405.43	38.50	0.00	97.56	0.646	66.34	9.2	65.407	E
D	1	59.00	358.65	98.50	0.00	220.80	0.267	63.03	4.1	15.046	B
	2	41.00	443.53	64.50	0.00	178.80	0.229	46.21	4.4	32.310	C

2038, PM WOD

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		55.29	E

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2038	PM WOD	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

		To				
		A	B	C	D	
16:00 - 16:15	From	A	0.00	0.00	92.00	66.00
		B	0.00	0.00	64.00	80.00
		C	78.00	45.00	0.00	0.00
		D	53.00	30.00	2.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:15 - 16:30	From	A	0.00	5.00	78.00	77.00
		B	0.00	0.00	41.00	33.00
		C	89.00	46.00	0.00	0.00
		D	68.00	21.00	4.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:30 - 16:45	From	A	0.00	0.00	79.00	58.00
		B	1.00	0.00	36.00	45.00
		C	72.00	60.00	0.00	0.00
		D	51.00	36.00	0.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:45 - 17:00	From	A	0.00	4.00	80.00	56.00
		B	5.00	0.00	52.00	58.00
		C	84.00	48.00	0.00	0.00
		D	81.00	47.00	6.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:00-16:15	A	158.00	158.00
	B	144.00	144.00
	C	123.00	123.00

	D	85.00	85.00
16:15-16:30	A	160.00	160.00
	B	74.00	74.00
	C	135.00	135.00
	D	93.00	93.00
16:30-16:45	A	137.00	137.00
	B	82.00	82.00
	C	132.00	132.00
	D	87.00	87.00
16:45-17:00	A	140.00	140.00
	B	115.00	115.00
	C	132.00	132.00
	D	134.00	134.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	0.75	54.17	20.7	D
B	0.96	81.49	23.2	F
C	0.69	59.74	18.6	E
D	0.35	23.88	11.4	C

Main Results for each time segment

16:00 - 16:15

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	92.00	466.25	58.50	0.00	170.47	0.540	81.25	10.8	43.729	D
	2	66.00	358.65	45.50	0.00	101.99	0.647	56.92	9.1	59.204	E
B	1	144.00	448.83	53.50	0.00	150.08	0.960	120.77	23.2	81.492	F
C	1	78.00	466.25	47.50	0.00	138.42	0.564	67.82	10.2	52.357	D
	2	45.00	405.43	34.50	0.00	87.42	0.515	38.40	6.6	61.642	E
D	1	53.00	358.65	102.50	0.00	229.76	0.231	49.57	3.4	12.826	B
	2	32.00	457.67	58.50	0.00	167.34	0.191	28.36	3.6	35.375	D

16:15 - 16:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	83.00	457.97	58.50	0.00	167.45	0.496	84.09	9.7	42.524	D
	2	77.00	358.65	45.50	0.00	101.99	0.755	75.00	11.1	66.732	E
B	1	74.00	442.01	53.50	0.00	147.80	0.501	88.17	9.1	49.171	D
C	1	89.00	466.25	47.50	0.00	138.42	0.643	87.38	11.8	55.662	E
	2	46.00	405.43	34.50	0.00	87.42	0.526	45.84	6.8	62.326	E
D	1	68.00	358.65	102.50	0.00	229.76	0.296	67.01	4.4	13.734	B
	2	25.00	444.90	58.50	0.00	162.66	0.154	25.80	2.8	34.716	C

16:30 - 16:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	79.00	466.25	58.50	0.00	170.47	0.463	79.50	9.2	41.519	D
	2	58.00	358.65	45.50	0.00	101.99	0.569	61.24	7.8	56.276	E
B	1	82.00	448.40	53.50	0.00	149.93	0.547	80.97	10.1	47.667	D
C	1	72.00	466.25	47.50	0.00	138.42	0.520	74.46	9.3	51.120	D
	2	60.00	405.43	34.50	0.00	87.42	0.686	57.54	9.2	70.092	E
D	1	51.00	358.65	102.50	0.00	229.76	0.222	52.12	3.3	12.718	B
	2	36.00	466.25	58.50	0.00	170.47	0.211	34.74	4.1	35.734	D

16:45 - 17:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	84.00	459.68	58.50	0.00	168.07	0.500	83.38	9.8	42.577	D
	2	56.00	358.65	45.50	0.00	101.99	0.549	56.31	7.5	55.081	E
B	1	115.00	445.62	53.50	0.00	149.00	0.772	110.02	15.1	58.687	E
C	1	84.00	466.25	47.50	0.00	138.42	0.607	82.29	11.0	54.091	D
	2	48.00	405.43	34.50	0.00	87.42	0.549	50.11	7.1	63.735	E
D	1	81.00	358.65	102.50	0.00	229.76	0.353	79.01	5.3	14.618	B
	2	53.00	450.94	58.50	0.00	164.87	0.321	51.03	6.1	38.023	D

Junctions 9

OSCADY 9 - Signalised Intersection Module

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The users of this computer program for the solution of an engineering problem are in no way relieved of their responsibility for the correctness of the solution

Filename: Sutton Cross 2038 WITH COMMITTED.j9
Path: C:\Users\Martin.Rogers\Documents\howth road
Report generation date: 10/05/2020 16:13:54

»2038, AM with committed

»2038, PM with committed

Summary of junction performance

	AM with committed				PM with committed			
	Queue (PCU)	Delay (s)	DOS	LOS	Queue (PCU)	Delay (s)	DOS	LOS
2038								
Arm A	47.9	106.98	1.03	F	23.7	59.64	0.83	E
Arm B	21.3	76.68	0.91	E	27.8	92.47	1.01	F
Arm C	38.6	98.93	1.14	F	21.9	61.93	0.75	E
Arm D	18.6	32.26	0.58	C	12.5	24.48	0.40	C

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle.

File summary

File Description

Title	Sutton Cross
Location	Howth
Site number	
Date	10/04/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Min	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	DOS Threshold	Average Delay threshold (s)	Queue threshold (PCU)
		0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2038	AM with committed	DIRECT	08:00	09:00	60	15
D2	2038	PM with committed	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2038, AM with committed

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		80.07	F

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description
A	Howth Road	
B	Carrickrock Road	
C	Dublin Road	
D	Station Road	

OSCADY Traffic Streams

Arm	Traffic Stream	Phase	Notional EEG (s)	Signals EEG (s)	Destination arms	Straight move
A	1	E	0.00	0.00	B, C	C
	2	D	0.00	0.00	D	C
B	1	F	0.00	0.00	A, C, D	D
C	1	B	0.00	1.00	A	A
	2	A	0.00	0.00	B	A
D	1	G	0.00	0.00	A	B

	2	C	0.00	5.00	B, C	B
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OSCADY Lanes

Arm	Traffic Stream	Destination arms	Gradient (%)	Width (m)	Turning radius (m)	Nearside lane	Has bay
A	1	B, C	0	2.50	5.00	✓	
	2	D	0	2.50	5.00	✓	
B	1	A, C, D	0	3.00	10.00	✓	
C	1	A	0	2.50		✓	
	2	B	0	2.50	10.00	✓	
D	1	A	0	2.50	5.00	✓	
	2	B, C	0	2.50	5.00	✓	

Signal Timings

Junction 1

Junction	Sequence to use	Cycle time (s)	Maximum cycle time (s)	Start displacement (s)	End displacement (s)
1	1	160	300	1.40	2.90

Optimisation options

Junction	Optimise stage lengths	Optimise cycle time	Optimiser demand source	Optimiser message
1	✓		Average	Timings provide delay minimisation.

Phases

Junction	Phase	Name	Minimum green (s)
1	A		15
	B		15
	C		15
	D		15
	E		15
	F		15
	G		15

Library Stages

Junction	Library Stage	Phases in stage	User stage minimum (s)	Run every N cycles	Probability of running (%)
1	1	B, E	7		
	2	D, E, G	7		
	3	C, G, F	7		
	4		7		
	5	A, B	7		

Losing / Gaining Phase Delays

Junction	Delay	Type	Phase	From stage	To stage	Relative delay (s)
1	1	Losing	B	1	2	1

Stage Sequences

Junction	Sequence	Name	Stage IDs	Stage ends
1	1		1, 2, 3, 4, 5	12, 53, 115, 123, 0

Intergreen Matrix for Junction 1

		To						
From		A	B	C	D	E	F	G
		A			5	5	5	5

	B			5	5		5	5
	C	0	0		0	5		
	D	0	0	0			5	
	E	0		0			0	
	F	0	0		0	5		
	G	0	0					

Interstage Matrix for Junction 1

		To				
		1	2	3	4	5
From	1	0	6	5	0	0
	2	0	0	5	0	0
	3	5	5	0	0	0
	4	0	0	0	0	0
	5	5	5	5	0	0

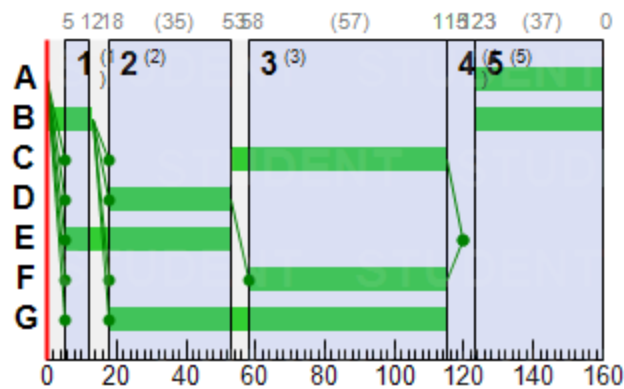
Resultant Stages

Junction	Resultant Stage	Library Stage ID	Phases in this stage	Stage start (s)	Stage end (s)	Stage duration (s)	User stage minimum (s)	Stage minimum (s)
1	1	1	B,E	5	12	7	7	7
	2	2	D,E,G	18	53	35	7	15
	3	3	C,G,F	58	115	57	7	15
	4	4		115	123	8	7	7
	5	5	A,B	123	0	37	7	15

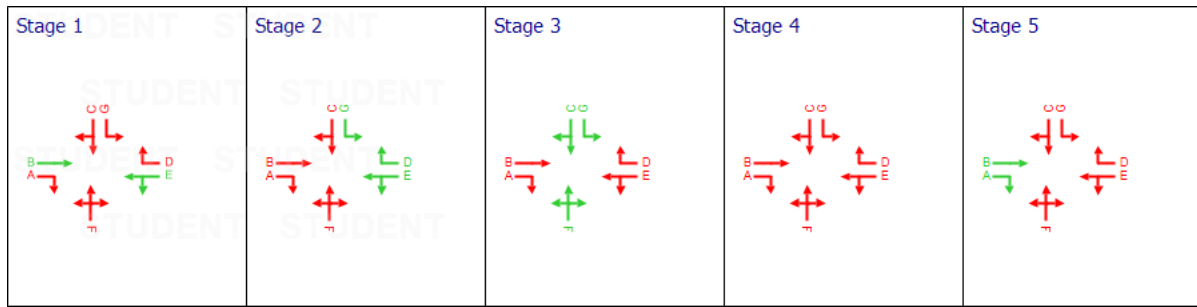
Resultant Phase Green Periods

Junction	Phase	Green period	Start time (s)	End time (s)	Duration (s)
1	A	1	123	0	37
	B	1	123	13	50
	C	1	53	115	62
	D	1	18	53	35
	E	1	5	53	48
	F	1	58	115	57
	G	1	18	115	97

Phase Timings Diagram for Junction 1



Stage Sequence Diagram for Junction 1



Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2038	AM with committed	DIRECT	08:00	09:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:00 - 08:15

		To			
		A	B	C	D
From	A	0.00	6.00	83.00	53.00
	B	0.00	0.00	63.00	74.00
	C	63.00	60.00	0.00	0.00
	D	62.00	72.00	4.00	0.00

Demand (PCU/TS)

08:15 - 08:30

		To			
		A	B	C	D
From	A	0.00	0.00	99.00	63.00
	B	2.00	0.00	87.00	56.00
	C	69.00	111.00	0.00	0.00
	D	56.00	91.00	12.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
08:30 - 08:45	From	A	0.00	4.00	143.00	84.00
		B	2.00	0.00	85.00	52.00
		C	97.00	72.00	0.00	0.00
		D	119.00	84.00	7.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
08:45 - 09:00	From	A	0.00	0.00	127.00	69.00
		B	1.00	0.00	79.00	68.00
		C	76.00	66.00	0.00	0.00
		D	63.00	36.00	7.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:00-08:15	A	142.00	142.00
	B	137.00	137.00
	C	123.00	123.00
	D	138.00	138.00
08:15-08:30	A	162.00	162.00
	B	145.00	145.00
	C	180.00	180.00
	D	159.00	159.00
08:30-08:45	A	231.00	231.00
	B	139.00	139.00
	C	169.00	169.00
	D	210.00	210.00
08:45-09:00	A	196.00	196.00
	B	148.00	148.00
	C	142.00	142.00
	D	106.00	106.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	1.03	106.98	47.9	F
B	0.91	76.68	21.3	E
C	1.14	98.93	38.6	F
D	0.58	32.26	18.6	C

Main Results for each time segment**08:00 - 08:15**

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	89.00	457.01	49.50	0.00	141.39	0.629	77.45	11.6	53.553	D
	2	53.00	358.65	36.50	0.00	81.82	0.648	45.05	8.0	66.978	E
B	1	137.00	447.86	58.50	0.00	163.75	0.837	119.26	17.7	60.113	E
C	1	63.00	466.25	51.50	0.00	150.07	0.420	55.22	7.8	45.099	D
	2	60.00	405.43	38.50	0.00	97.56	0.615	51.33	8.7	62.422	E
D	1	62.00	358.65	98.50	0.00	220.80	0.281	57.70	4.3	15.240	B
	2	76.00	459.00	63.50	0.00	182.17	0.417	67.67	8.3	36.971	D

08:15 - 08:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	99.00	466.25	49.50	0.00	144.25	0.686	97.53	13.0	56.328	E
	2	63.00	358.65	36.50	0.00	81.82	0.770	60.92	10.0	76.680	E
B	1	145.00	438.39	58.50	0.00	160.29	0.905	142.38	20.4	71.276	E
C	1	69.00	466.25	51.50	0.00	150.07	0.460	68.22	8.6	46.215	D
	2	111.00	405.43	38.50	0.00	97.56	1.138	89.63	30.0	131.694	F
D	1	56.00	358.65	98.50	0.00	220.80	0.254	56.42	3.9	14.839	B
	2	103.00	450.50	63.50	0.00	178.79	0.576	99.82	11.5	41.729	D

08:30 - 08:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	147.00	462.47	49.50	0.00	143.08	1.027	131.35	28.7	98.657	F
	2	84.00	358.65	36.50	0.00	81.82	1.027	74.80	19.2	119.218	F
B	1	139.00	437.66	58.50	0.00	160.02	0.869	139.92	19.4	70.915	E
C	1	97.00	466.25	51.50	0.00	150.07	0.646	93.17	12.4	52.732	D
	2	72.00	405.43	38.50	0.00	97.56	0.738	88.75	13.3	148.678	F
D	1	119.00	358.65	98.50	0.00	220.80	0.539	114.37	8.5	20.494	C
	2	91.00	455.73	63.50	0.00	180.87	0.503	92.44	10.1	39.430	D

08:45 - 09:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	127.00	466.25	49.50	0.00	144.25	0.880	134.32	21.4	99.491	F

	2	69.00	358.65	36.50	0.00	81.82	0.843	74.60	13.6	120.751	F
B	1	148.00	442.84	58.50	0.00	161.91	0.914	146.09	21.3	76.680	E
C	1	76.00	466.25	51.50	0.00	150.07	0.506	78.91	9.5	47.737	D
	2	66.00	405.43	38.50	0.00	97.56	0.677	69.49	9.8	67.625	E
D	1	63.00	358.65	98.50	0.00	220.80	0.285	67.13	4.4	15.324	B
	2	43.00	444.54	63.50	0.00	176.43	0.244	48.41	4.7	33.214	C

2038, PM with committed

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		59.69	E

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2038	PM with committed	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

		To				
		A	B	C	D	
16:00 - 16:15	From	A	0.00	0.00	100.00	71.00
		B	0.00	0.00	69.00	83.00
		C	94.00	50.00	0.00	0.00
		D	64.00	33.00	2.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:15 - 16:30	From	A	0.00	5.00	87.00	83.00
		B	0.00	0.00	46.00	36.00
		C	106.00	51.00	0.00	0.00
		D	79.00	24.00	5.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:30 - 16:45	From	A	0.00	0.00	88.00	63.00
		B	1.00	0.00	41.00	48.00
		C	89.00	65.00	0.00	0.00
		D	61.00	39.00	0.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:45 - 17:00	From	A	0.00	4.00	88.00	61.00
		B	5.00	0.00	57.00	61.00
		C	101.00	53.00	0.00	0.00
		D	91.00	50.00	6.00	0.00

Vehicle Mix**Heavy Vehicle Percentages**

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data**Demand for each time segment**

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:00-16:15	A	171.00	171.00
	B	152.00	152.00
	C	144.00	144.00

	D	99.00	99.00
16:15-16:30	A	175.00	175.00
	B	82.00	82.00
	C	157.00	157.00
	D	108.00	108.00
16:30-16:45	A	151.00	151.00
	B	90.00	90.00
	C	154.00	154.00
	D	100.00	100.00
16:45-17:00	A	153.00	153.00
	B	123.00	123.00
	C	154.00	154.00
	D	147.00	147.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	0.83	59.64	23.7	E
B	1.01	92.47	27.8	F
C	0.75	61.93	21.9	E
D	0.40	24.48	12.5	C

Main Results for each time segment

16:00 - 16:15

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	100.00	466.25	57.50	0.00	167.56	0.597	88.09	11.9	46.400	D
	2	71.00	358.65	44.50	0.00	99.75	0.712	60.90	10.1	63.873	E
B	1	152.00	448.23	53.50	0.00	149.88	1.014	124.16	27.8	92.473	F
C	1	94.00	466.25	48.50	0.00	141.33	0.665	81.59	12.4	55.787	E
	2	50.00	405.43	35.50	0.00	89.96	0.556	42.68	7.3	62.386	E
D	1	64.00	358.65	101.50	0.00	227.52	0.281	59.77	4.2	13.942	B
	2	35.00	458.39	58.50	0.00	167.60	0.209	31.02	4.0	35.703	D

16:15 - 16:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	92.00	458.77	57.50	0.00	164.87	0.558	93.00	10.9	45.256	D
	2	83.00	358.65	44.50	0.00	99.75	0.832	80.34	12.8	75.580	E
B	1	82.00	441.59	53.50	0.00	147.66	0.555	98.56	11.3	63.789	E
C	1	106.00	466.25	48.50	0.00	141.33	0.750	104.00	14.4	60.909	E
	2	51.00	405.43	35.50	0.00	89.96	0.567	50.83	7.5	63.195	E
D	1	79.00	358.65	101.50	0.00	227.52	0.347	77.98	5.2	14.972	B
	2	29.00	443.32	58.50	0.00	162.09	0.179	29.69	3.3	35.176	D

16:30 - 16:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	88.00	466.25	57.50	0.00	167.56	0.525	88.53	10.4	44.073	D
	2	63.00	358.65	44.50	0.00	99.75	0.632	67.00	8.8	60.821	E
B	1	90.00	447.43	53.50	0.00	149.61	0.602	90.09	11.2	49.724	D
C	1	89.00	466.25	48.50	0.00	141.33	0.630	91.72	11.7	54.835	D
	2	65.00	405.43	35.50	0.00	89.96	0.723	62.45	10.0	71.651	E
D	1	61.00	358.65	101.50	0.00	227.52	0.268	62.22	4.0	13.759	B
	2	39.00	466.25	58.50	0.00	170.47	0.229	37.86	4.4	36.070	D

16:45 - 17:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	92.00	460.25	57.50	0.00	165.40	0.556	91.48	10.9	45.088	D
	2	61.00	358.65	44.50	0.00	99.75	0.612	61.34	8.4	58.926	E
B	1	123.00	445.10	53.50	0.00	148.83	0.826	117.52	16.7	63.304	E
C	1	101.00	466.25	48.50	0.00	141.33	0.715	99.14	13.5	58.650	E
	2	53.00	405.43	35.50	0.00	89.96	0.589	55.19	7.9	64.856	E
D	1	91.00	358.65	101.50	0.00	227.52	0.400	88.95	6.1	15.900	B
	2	56.00	451.73	58.50	0.00	165.16	0.339	54.02	6.4	38.415	D

Junctions 9

OSCADY 9 - Signalised Intersection Module

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The users of this computer program for the solution of an engineering problem are in no way relieved of their responsibility for the correctness of the solution

Filename: Sutton Cross 2038 WITH COMMITTED PLUS DEV.j9

Path: C:\Users\Martin.Rogers\Documents\howth road

Report generation date: 10/05/2020 16:32:30

»2038, AM with committed and dev

»2038, PM with committed and dev

Summary of junction performance

	AM with committed and dev				PM with committed and dev			
	Queue (PCU)	Delay (s)	DOS	LOS	Queue (PCU)	Delay (s)	DOS	LOS
2038								
Arm A	52.1	123.90	1.06	F	23.8	59.73	0.83	E
Arm B	21.0	74.04	0.91	E	29.3	96.63	1.03	F
Arm C	40.7	108.40	1.17	F	22.1	60.82	0.75	E
Arm D	18.3	31.38	0.57	C	12.9	25.25	0.41	C

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle.

File summary

File Description

Title	Sutton Cross
Location	Howth
Site number	
Date	10/04/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Min	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	DOS Threshold	Average Delay threshold (s)	Queue threshold (PCU)
		0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2038	AM with committed and dev	DIRECT	08:00	09:00	60	15
D2	2038	PM with committed and dev	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2038, AM with committed and dev

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		86.68	F

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description
A	Howth Road	
B	Carrickrock Road	
C	Dublin Road	
D	Station Road	

OSCADY Traffic Streams

Arm	Traffic Stream	Phase	Notional EEG (s)	Signals EEG (s)	Destination arms	Straight move
A	1	E	0.00	0.00	B, C	C
	2	D	0.00	0.00	D	C
B	1	F	0.00	0.00	A, C, D	D
C	1	B	0.00	1.00	A	A
	2	A	0.00	0.00	B	A
D	1	G	0.00	0.00	A	B

	2	C	0.00	5.00	B, C	B
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OSCADY Lanes

Arm	Traffic Stream	Destination arms	Gradient (%)	Width (m)	Turning radius (m)	Nearside lane	Has bay
A	1	B, C	0	2.50	5.00	✓	
	2	D	0	2.50	5.00	✓	
B	1	A, C, D	0	3.00	10.00	✓	
C	1	A	0	2.50		✓	
	2	B	0	2.50	10.00	✓	
D	1	A	0	2.50	5.00	✓	
	2	B, C	0	2.50	5.00	✓	

Signal Timings

Junction 1

Junction	Sequence to use	Cycle time (s)	Maximum cycle time (s)	Start displacement (s)	End displacement (s)
1	1	160	300	1.40	2.90

Optimisation options

Junction	Optimise stage lengths	Optimise cycle time	Optimiser demand source	Optimiser message
1	✓		Average	Timings provide delay minimisation.

Phases

Junction	Phase	Name	Minimum green (s)
1	A		15
	B		15
	C		15
	D		15
	E		15
	F		15
	G		15

Library Stages

Junction	Library Stage	Phases in stage	User stage minimum (s)	Run every N cycles	Probability of running (%)
1	1	B, E	7		
	2	D, E, G	7		
	3	C, G, F	7		
	4		7		
	5	A, B	7		

Losing / Gaining Phase Delays

Junction	Delay	Type	Phase	From stage	To stage	Relative delay (s)
1	1	Losing	B	1	2	1

Stage Sequences

Junction	Sequence	Name	Stage IDs	Stage ends
1	1		1, 2, 3, 4, 5	12, 53, 116, 124, 0

Intergreen Matrix for Junction 1

		To						
From		A	B	C	D	E	F	G
		A			5	5	5	5

	B			5	5		5	5
	C	0	0		0	5		
	D	0	0	0			5	
	E	0		0			0	
	F	0	0		0	5		
	G	0	0					

Interstage Matrix for Junction 1

		To				
		1	2	3	4	5
From	1	0	6	5	0	0
	2	0	0	5	0	0
	3	5	5	0	0	0
	4	0	0	0	0	0
	5	5	5	5	0	0

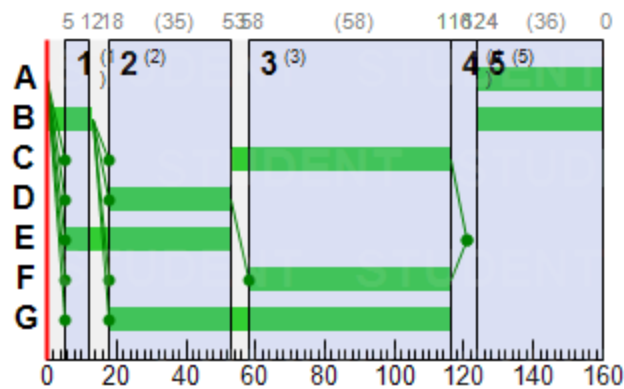
Resultant Stages

Junction	Resultant Stage	Library Stage ID	Phases in this stage	Stage start (s)	Stage end (s)	Stage duration (s)	User stage minimum (s)	Stage minimum (s)
1	1	1	B,E	5	12	7	7	7
	2	2	D,E,G	18	53	35	7	15
	3	3	C,G,F	58	116	58	7	15
	4	4		116	124	8	7	7
	5	5	A,B	124	0	36	7	15

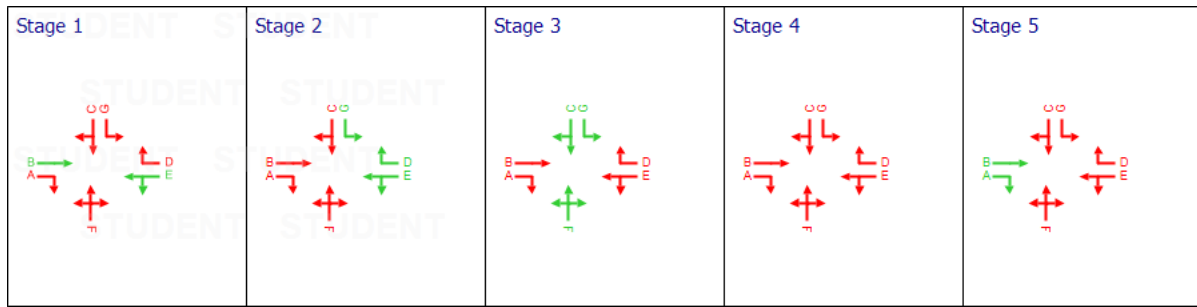
Resultant Phase Green Periods

Junction	Phase	Green period	Start time (s)	End time (s)	Duration (s)
1	A	1	124	0	36
	B	1	124	13	49
	C	1	53	116	63
	D	1	18	53	35
	E	1	5	53	48
	F	1	58	116	58
	G	1	18	116	98

Phase Timings Diagram for Junction 1



Stage Sequence Diagram for Junction 1



Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2038	AM with committed and dev	DIRECT	08:00	09:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:00 - 08:15

		To			
		A	B	C	D
From	A	0.00	6.00	86.00	54.00
	B	0.00	0.00	64.00	74.00
	C	64.00	60.00	0.00	0.00
	D	62.00	73.00	4.00	0.00

Demand (PCU/TS)

08:15 - 08:30

		To			
		A	B	C	D
From	A	0.00	0.00	103.00	64.00
	B	2.00	0.00	88.00	57.00
	C	70.00	111.00	0.00	0.00
	D	57.00	91.00	12.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
08:30 - 08:45	From	A	0.00	4.00	147.00	86.00
		B	2.00	0.00	86.00	53.00
		C	98.00	73.00	0.00	0.00
		D	119.00	84.00	7.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
08:45 - 09:00	From	A	0.00	0.00	131.00	70.00
		B	1.00	0.00	80.00	68.00
		C	76.00	66.00	0.00	0.00
		D	64.00	37.00	7.00	0.00

Vehicle Mix**Heavy Vehicle Percentages**

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data**Demand for each time segment**

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:00-08:15	A	146.00	146.00
	B	138.00	138.00
	C	124.00	124.00
	D	139.00	139.00
08:15-08:30	A	167.00	167.00
	B	147.00	147.00
	C	181.00	181.00
	D	160.00	160.00
08:30-08:45	A	237.00	237.00
	B	141.00	141.00
	C	171.00	171.00
	D	210.00	210.00
08:45-09:00	A	201.00	201.00
	B	149.00	149.00
	C	142.00	142.00
	D	108.00	108.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	1.06	123.90	52.1	F
B	0.91	74.04	21.0	E
C	1.17	108.40	40.7	F
D	0.57	31.38	18.3	C

Main Results for each time segment**08:00 - 08:15**

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	92.00	457.30	49.50	0.00	141.48	0.650	80.00	12.0	54.468	D
	2	54.00	358.65	36.50	0.00	81.82	0.660	45.86	8.1	67.694	E
B	1	138.00	447.61	59.50	0.00	166.46	0.829	120.42	17.6	58.643	E
C	1	64.00	466.25	50.50	0.00	147.16	0.435	56.01	8.0	46.207	D
	2	60.00	405.43	37.50	0.00	95.02	0.631	51.21	8.8	64.077	E
D	1	62.00	358.65	99.50	0.00	223.04	0.278	57.77	4.2	14.756	B
	2	77.00	459.10	64.50	0.00	185.07	0.416	68.65	8.3	36.298	D

08:15 - 08:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	103.00	466.25	49.50	0.00	144.25	0.714	101.33	13.7	57.875	E
	2	64.00	358.65	36.50	0.00	81.82	0.782	61.86	10.3	77.940	E
B	1	147.00	438.48	59.50	0.00	163.06	0.902	144.26	20.3	69.691	E
C	1	70.00	466.25	50.50	0.00	147.16	0.476	69.21	8.8	47.387	D
	2	111.00	405.43	37.50	0.00	95.02	1.168	87.83	32.0	138.951	F
D	1	57.00	358.65	99.50	0.00	223.04	0.256	57.35	3.9	14.432	B
	2	103.00	450.50	64.50	0.00	181.61	0.567	99.98	11.4	40.755	D

08:30 - 08:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	151.00	462.57	49.50	0.00	143.11	1.055	133.23	31.4	104.248	F
	2	86.00	358.65	36.50	0.00	81.82	1.051	75.66	20.6	124.024	F
B	1	141.00	437.77	59.50	0.00	162.79	0.866	141.91	19.4	69.329	E
C	1	98.00	466.25	50.50	0.00	147.16	0.666	94.08	12.7	54.376	D
	2	73.00	405.43	37.50	0.00	95.02	0.768	90.60	14.4	180.922	F
D	1	119.00	358.65	99.50	0.00	223.04	0.534	114.52	8.4	19.837	B
	2	91.00	455.73	64.50	0.00	183.72	0.495	92.42	10.0	38.537	D

08:45 - 09:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	131.00	466.25	49.50	0.00	144.25	0.908	137.96	24.5	118.813	F

	2	70.00	358.65	36.50	0.00	81.82	0.856	76.12	14.5	133.427	F
B	1	149.00	442.65	59.50	0.00	164.61	0.905	147.45	21.0	74.044	E
C	1	76.00	466.25	50.50	0.00	147.16	0.516	79.11	9.6	48.794	D
	2	66.00	405.43	37.50	0.00	95.02	0.695	70.38	10.0	70.341	E
D	1	64.00	358.65	99.50	0.00	223.04	0.287	67.99	4.4	14.906	B
	2	44.00	445.01	64.50	0.00	179.40	0.245	49.23	4.7	32.616	C

2038, PM with committed and dev

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	Signalised		60.28	E

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2038	PM with committed and dev	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000
D		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

		To				
		A	B	C	D	
16:00 - 16:15	From	A	0.00	0.00	101.00	72.00
		B	0.00	0.00	69.00	83.00
		C	97.00	51.00	0.00	0.00
		D	65.00	34.00	2.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:15 - 16:30	From	A	0.00	5.00	88.00	83.00
		B	0.00	0.00	46.00	36.00
		C	108.00	52.00	0.00	0.00
		D	80.00	24.00	5.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:30 - 16:45	From	A	0.00	0.00	88.00	64.00
		B	1.00	0.00	41.00	48.00
		C	91.00	66.00	0.00	0.00
		D	63.00	40.00	0.00	0.00

Demand (PCU/TS)

		To				
		A	B	C	D	
16:45 - 17:00	From	A	0.00	4.00	89.00	62.00
		B	5.00	0.00	57.00	61.00
		C	103.00	54.00	0.00	0.00
		D	93.00	51.00	6.00	0.00

Vehicle Mix**Heavy Vehicle Percentages**

		To			
		A	B	C	D
From	A	0	0	0	0
	B	0	0	0	0
	C	0	0	0	0
	D	0	0	0	0

Detailed Demand Data**Demand for each time segment**

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:00-16:15	A	173.00	173.00
	B	152.00	152.00
	C	148.00	148.00

	D	101.00	101.00
16:15-16:30	A	176.00	176.00
	B	82.00	82.00
	C	160.00	160.00
	D	109.00	109.00
16:30-16:45	A	152.00	152.00
	B	90.00	90.00
	C	157.00	157.00
	D	103.00	103.00
16:45-17:00	A	155.00	155.00
	B	123.00	123.00
	C	157.00	157.00
	D	150.00	150.00

Results

Results Summary for whole modelled period

Arm	Max DOS	Max Delay (s)	Max Queue (PCU)	Max LOS
A	0.83	59.73	23.8	E
B	1.03	96.63	29.3	F
C	0.75	60.82	22.1	E
D	0.41	25.25	12.9	C

Main Results for each time segment

16:00 - 16:15

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	101.00	466.25	57.50	0.00	167.56	0.603	88.96	12.0	46.624	D
	2	72.00	358.65	44.50	0.00	99.75	0.722	61.71	10.3	64.556	E
B	1	152.00	448.23	52.50	0.00	147.08	1.033	122.68	29.3	96.634	F
C	1	97.00	466.25	49.50	0.00	144.25	0.672	84.29	12.7	55.386	E
	2	51.00	405.43	36.50	0.00	92.49	0.551	43.61	7.4	61.355	E
D	1	65.00	358.65	100.50	0.00	225.28	0.289	60.63	4.4	14.479	B
	2	36.00	458.61	57.50	0.00	164.81	0.218	31.86	4.1	36.540	D

16:15 - 16:30

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	93.00	458.85	57.50	0.00	164.90	0.564	94.01	11.0	45.470	D
	2	83.00	358.65	44.50	0.00	99.75	0.832	80.53	12.8	75.717	E
B	1	82.00	441.59	52.50	0.00	144.90	0.566	99.72	11.6	69.617	E
C	1	108.00	466.25	49.50	0.00	144.25	0.749	106.18	14.5	60.059	E
	2	52.00	405.43	36.50	0.00	92.49	0.562	51.84	7.6	62.110	E
D	1	80.00	358.65	100.50	0.00	225.28	0.355	78.96	5.4	15.553	B
	2	29.00	443.32	57.50	0.00	159.32	0.182	29.81	3.3	35.886	D

16:30 - 16:45

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	88.00	466.25	57.50	0.00	167.56	0.525	88.66	10.4	44.078	D
	2	64.00	358.65	44.50	0.00	99.75	0.642	67.84	8.9	61.443	E
B	1	90.00	447.43	52.50	0.00	146.81	0.613	90.28	11.3	50.923	D
C	1	91.00	466.25	49.50	0.00	144.25	0.631	93.69	11.8	54.118	D
	2	66.00	405.43	36.50	0.00	92.49	0.714	63.50	10.1	70.053	E
D	1	63.00	358.65	100.50	0.00	225.28	0.280	64.18	4.2	14.356	B
	2	40.00	466.25	57.50	0.00	167.56	0.239	38.73	4.6	36.920	D

16:45 - 17:00

Arm	Traffic Stream	Total Demand (PCU/TS)	Calculated saturation flow (PCU/TS)	Effective green time (s)	NEEG (s)	Capacity (PCU/TS)	DOS	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Signalised level of service
A	1	93.00	460.31	57.50	0.00	165.42	0.562	92.36	11.0	45.290	D
	2	62.00	358.65	44.50	0.00	99.75	0.622	62.34	8.6	59.484	E
B	1	123.00	445.10	52.50	0.00	146.05	0.842	117.27	17.1	65.700	E
C	1	103.00	466.25	49.50	0.00	144.25	0.714	101.16	13.7	57.840	E
	2	54.00	405.43	36.50	0.00	92.49	0.584	56.14	7.9	63.647	E
D	1	93.00	358.65	100.50	0.00	225.28	0.413	90.91	6.3	16.607	B
	2	57.00	451.98	57.50	0.00	162.43	0.351	55.00	6.6	39.353	D

APPENDIX 4B

Development Entrance Priority junction

Junctions 9
PICADY 9 - Priority Intersection Module
For sales and distribution information, program advice and maintenance, contact TRL: +44 (0)1344 379777 software@trl.co.uk www.trlsoftware.co.uk
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Filename: Howth Dev Entrance 2023.j9
Path: C:\Users\Martin.Rogers\Documents\howth road
Report generation date: 11/05/2020 14:23:47

»2023 , AM
 »2023, PM

Summary of junction performance

	AM					PM				
	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity
2023										
Stream B-AC	0.1	9.93	0.08	A	91 %	0.0	9.24	0.02	A	123 %
Stream C-B	0.0	7.86	0.01	A	[Stream B-AC]	0.0	7.65	0.03	A	[Stream B-AC]

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle. Network Residual Capacity indicates the amount by which network flow could be increased before a user-definable threshold (see Analysis Options) is met.

File summary

File Description

Title	Howth SHD
Location	Howth Road Dublin 13
Site number	
Date	11/05/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Hour	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	Residual capacity criteria type	RFC Threshold	Average Delay threshold (s)	Queue threshold (PCU)
	✓	Delay	0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM	DIRECT	08:00	09:00	60	15
D2	2023	PM	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2023 , AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		0.34	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	91	Stream B-AC

Arms

Arms

Arm	Name	Description	Arm type
A	Howth Road East		Major
B	Development Entrance		Minor
C	Howth Road West		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Width for right turn (m)	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
C	7.30		✓	2.20	50.0		-

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
B	One lane	3.00	50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/TS)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	129.627	0.089	0.225	0.142	0.322
1	B-C	163.853	0.095	0.240	-	-
1	C-B	150.730	0.220	0.220	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.
Streams may be combined, in which case capacity will be adjusted.
Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM	DIRECT	08:00	09:00	60	15

Vehicle mix varies over time	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:00 - 08:15

		To		
		A	B	C
From	A	0.00	1.00	117.00
	B	3.00	0.00	5.00
	C	86.00	1.00	0.00

Demand (PCU/TS)

08:15 - 08:30

		To		
		A	B	C
From	A	0.00	1.00	127.00
	B	3.00	0.00	5.00
	C	86.00	1.00	0.00

Demand (PCU/TS)

08:30 - 08:45

		To		
		A	B	C
From	A	0.00	1.00	159.00
	B	3.00	0.00	5.00
	C	147.00	1.00	0.00

Demand (PCU/TS)

08:45 - 09:00	From	To			
			A	B	C
		A	0.00	1.00	136.00
		B	3.00	0.00	5.00
	C	137.00	1.00	0.00	

Vehicle Mix

Heavy Vehicle Percentages

08:00 - 08:15	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:15 - 08:30	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:30 - 08:45	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:45 - 09:00	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.08	9.93	0.1	A
C-A				
C-B	0.01	7.86	0.0	A
A-B				
A-C				

Main Results for each time segment**08:00 - 08:15**

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	114.41	0.070	7.93	0.1	8.445	A
C-A	86.00			86.00			
C-B	1.00	124.72	0.008	0.99	0.0	7.273	A
A-B	1.00			1.00			
A-C	117.00			117.00			

08:15 - 08:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	112.00	0.071	8.00	0.1	8.653	A
C-A	86.00			86.00			
C-B	1.00	122.52	0.008	1.00	0.0	7.405	A
A-B	1.00			1.00			
A-C	127.00			127.00			

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	98.62	0.081	7.99	0.1	9.929	A
C-A	147.00			147.00			
C-B	1.00	115.47	0.009	1.00	0.0	7.862	A
A-B	1.00			1.00			
A-C	159.00			159.00			

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	105.25	0.076	8.00	0.1	9.256	A
C-A	137.00			137.00			
C-B	1.00	120.54	0.008	1.00	0.0	7.528	A
A-B	1.00			1.00			
A-C	136.00			136.00			

2023, PM**Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network**Junctions**

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		0.19	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	123	Stream B-AC

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2023	PM	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:00 - 16:15

		To		
		A	B	C
From	A	0.00	2.00	123.00
	B	1.00	0.00	1.00
	C	144.00	4.00	0.00

Demand (PCU/TS)

16:15 - 16:30

		To		
		A	B	C
From	A	0.00	2.00	111.00
	B	1.00	0.00	1.00
	C	134.00	4.00	0.00

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	2.00	114.00
	B	1.00	0.00	1.00
	C	132.00	4.00	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	2.00	130.00
	B	1.00	0.00	1.00
	C	137.00	4.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

From	To		
	A	B	C
A	0	0	0
B	0	0	0
C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.02	9.24	0.0	A
C-A				
C-B	0.03	7.65	0.0	A
A-B				
A-C				

Main Results for each time segment

16:00 - 16:15

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	2.00	100.29	0.020	1.98	0.0	9.151	A
C-A	144.00			144.00			
C-B	4.00	123.18	0.032	3.97	0.0	7.547	A
A-B	2.00			2.00			
A-C	123.00			123.00			

16:15 - 16:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	2.00	104.29	0.019	2.00	0.0	8.797	A
C-A	134.00			134.00			
C-B	4.00	125.83	0.032	4.00	0.0	7.389	A
A-B	2.00			2.00			
A-C	111.00			111.00			

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	2.00	103.79	0.019	2.00	0.0	8.843	A
C-A	132.00			132.00			
C-B	4.00	125.16	0.032	4.00	0.0	7.426	A
A-B	2.00			2.00			
A-C	114.00			114.00			

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	2.00	99.35	0.020	2.00	0.0	9.244	A
C-A	137.00			137.00			
C-B	4.00	121.64	0.033	4.00	0.0	7.649	A
A-B	2.00			2.00			
A-C	130.00			130.00			

Junctions 9
PICADY 9 - Priority Intersection Module
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Filename: Howth Dev Entrance 2038.j9
Path: C:\Users\Martin.Rogers\Documents\howth road
Report generation date: 11/05/2020 14:59:46

»2038 , AM
 »2038, PM

Summary of junction performance

	AM					PM				
	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity
2038										
Stream B-AC	0.1	10.81	0.09	B	70 %	0.0	9.91	0.02	A	97 %
Stream C-B	0.0	8.19	0.01	A	[Stream B-AC]	0.0	7.93	0.03	A	[Stream B-AC]

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle. Network Residual Capacity indicates the amount by which network flow could be increased before a user-definable threshold (see Analysis Options) is met.

File summary

File Description

Title	Howth SHD
Location	Howth Road Dublin 13
Site number	
Date	11/05/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Hour	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	Residual capacity criteria type	RFC Threshold	Average Delay threshold (s)	Queue threshold (PCU)
	✓	Delay	0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2038	AM	DIRECT	08:00	09:00	60	15
D2	2038	PM	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2038 , AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		0.32	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	70	Stream B-AC

Arms

Arms

Arm	Name	Description	Arm type
A	Howth Road East		Major
B	Development Entrance		Minor
C	Howth Road West		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Width for right turn (m)	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
C	7.30		✓	2.20	50.0		-

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
B	One lane	3.00	50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/TS)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	129.627	0.089	0.225	0.142	0.322
1	B-C	163.853	0.095	0.240	-	-
1	C-B	150.730	0.220	0.220	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments. Streams may be combined, in which case capacity will be adjusted. Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2038	AM	DIRECT	08:00	09:00	60	15

Vehicle mix varies over time	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

		To			
		A	B	C	
08:00 - 08:15	From	A	0.00	1.00	131.00
		B	3.00	0.00	5.00
		C	98.00	1.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	
08:15 - 08:30	From	A	0.00	1.00	142.00
		B	3.00	0.00	5.00
		C	99.00	1.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	
08:30 - 08:45	From	A	0.00	1.00	180.00
		B	3.00	0.00	5.00
		C	168.00	1.00	0.00

Demand (PCU/TS)

08:45 - 09:00	From	To			
			A	B	C
		A	0.00	1.00	153.00
		B	3.00	0.00	5.00
	C	157.00	1.00	0.00	

Vehicle Mix

Heavy Vehicle Percentages

08:00 - 08:15	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:15 - 08:30	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:30 - 08:45	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:45 - 09:00	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.09	10.81	0.1	B
C-A				
C-B	0.01	8.19	0.0	A
A-B				
A-C				

Main Results for each time segment**08:00 - 08:15**

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	110.00	0.073	7.92	0.1	8.810	A
C-A	98.00			98.00			
C-B	1.00	121.64	0.008	0.99	0.0	7.459	A
A-B	1.00			1.00			
A-C	131.00			131.00			

08:15 - 08:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	107.25	0.075	8.00	0.1	9.067	A
C-A	99.00			99.00			
C-B	1.00	119.21	0.008	1.00	0.0	7.612	A
A-B	1.00			1.00			
A-C	142.00			142.00			

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	91.23	0.088	7.98	0.1	10.811	B
C-A	168.00			168.00			
C-B	1.00	110.84	0.009	1.00	0.0	8.193	A
A-B	1.00			1.00			
A-C	180.00			180.00			

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	99.12	0.081	8.01	0.1	9.878	A
C-A	157.00			157.00			
C-B	1.00	116.79	0.009	1.00	0.0	7.772	A
A-B	1.00			1.00			
A-C	153.00			153.00			

2038, PM**Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network**Junctions**

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		0.17	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	97	Stream B-AC

Traffic Demand**Demand Set Details**

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2038	PM	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data**Demand (PCU/TS)**

		To		
		A	B	C
From	A	0.00	2.00	141.00
	B	1.00	0.00	1.00
	C	162.00	4.00	0.00

16:00 - 16:15

Demand (PCU/TS)

		To		
		A	B	C
From	A	0.00	2.00	126.00
	B	1.00	0.00	1.00
	C	151.00	4.00	0.00

16:15 - 16:30

Demand (PCU/TS)

		To		
		A	B	C
From	A	0.00	2.00	130.00
	B	1.00	0.00	1.00
	C	149.00	4.00	0.00

16:30 - 16:45

Demand (PCU/TS)

		To		
		A	B	C
From	A	0.00	2.00	149.00
	B	1.00	0.00	1.00
	C	154.00	4.00	0.00

16:45 - 17:00

Vehicle Mix

Heavy Vehicle Percentages

	To			
	A	B	C	
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.02	9.91	0.0	A
C-A				
C-B	0.03	7.93	0.0	A
A-B				
A-C				

Main Results for each time segment

16:00 - 16:15

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	2.00	93.84	0.021	1.98	0.0	9.795	A
C-A	162.00			162.00			
C-B	4.00	119.21	0.034	3.97	0.0	7.806	A
A-B	2.00			2.00			
A-C	141.00			141.00			

16:15 - 16:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	2.00	98.76	0.020	2.00	0.0	9.302	A
C-A	151.00			151.00			
C-B	4.00	122.52	0.033	4.00	0.0	7.595	A
A-B	2.00			2.00			
A-C	126.00			126.00			

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	2.00	98.01	0.020	2.00	0.0	9.373	A
C-A	149.00			149.00			
C-B	4.00	121.64	0.033	4.00	0.0	7.649	A
A-B	2.00			2.00			
A-C	130.00			130.00			

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	2.00	92.78	0.022	2.00	0.0	9.913	A
C-A	154.00			154.00			
C-B	4.00	117.45	0.034	4.00	0.0	7.932	A
A-B	2.00			2.00			
A-C	149.00			149.00			

APPENDIX 4C

HOWTH ROAD / CHURCH ROAD PRIORITY JUNCTION

Junctions 9
PICADY 9 - Priority Intersection Module
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Filename: Howth Rd Church Rd 2023.j9
Path: C:\Users\Martin.Rogers\Documents\howth road
Report generation date: 11/05/2020 20:00:29

»2023 , AM
 »2023, PM

Summary of junction performance

	AM					PM				
	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity
2023										
Stream B-AC	1.1	16.61	0.54	C	27 %	0.2	8.37	0.14	A	116 %
Stream C-B	0.4	10.24	0.29	B	[Stream B-AC]	0.3	9.14	0.21	A	[Stream B-AC]

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle. Network Residual Capacity indicates the amount by which network flow could be increased before a user-definable threshold (see Analysis Options) is met.

File summary

File Description

Title	Howth SHD
Location	Howth Road Dublin 13
Site number	
Date	11/05/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Hour	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	Residual capacity criteria type	RFC Threshold	Average Delay threshold (s)	Queue threshold (PCU)
	✓	Delay	0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM	DIRECT	08:00	09:00	60	15
D2	2023	PM	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2023 , AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		2.96	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	27	Stream B-AC

Arms

Arms

Arm	Name	Description	Arm type
A	Howth Road East		Major
B	Church Road		Minor
C	Howth Road West		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Width for right turn (m)	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
C	7.30		✓	2.20	50.0		-

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
B	One lane	3.00	50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/TS)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	129.627	0.089	0.225	0.142	0.322
1	B-C	163.853	0.095	0.240	-	-
1	C-B	150.730	0.220	0.220	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.
Streams may be combined, in which case capacity will be adjusted.
Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM	DIRECT	08:00	09:00	60	15

Vehicle mix varies over time	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:00 - 08:15

		To		
		A	B	C
From	A	0.00	3.00	122.00
	B	2.00	0.00	17.00
	C	77.00	23.00	0.00

Demand (PCU/TS)

08:15 - 08:30

		To		
		A	B	C
From	A	0.00	5.00	123.00
	B	3.00	0.00	48.00
	C	84.00	35.00	0.00

Demand (PCU/TS)

08:30 - 08:45

		To		
		A	B	C
From	A	0.00	7.00	158.00
	B	5.00	0.00	58.00
	C	156.00	26.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	
08:45 - 09:00	From	A	0.00	15.00	147.00
		B	1.00	0.00	31.00
		C	109.00	15.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

		To			
		A	B	C	
08:00 - 08:15	From	A	0	0	0
		B	0	0	0
		C	0	0	0

Heavy Vehicle Percentages

		To			
		A	B	C	
08:15 - 08:30	From	A	0	0	0
		B	0	0	0
		C	0	0	0

Heavy Vehicle Percentages

		To			
		A	B	C	
08:30 - 08:45	From	A	0	0	0
		B	0	0	0
		C	0	0	0

Heavy Vehicle Percentages

		To			
		A	B	C	
08:45 - 09:00	From	A	0	0	0
		B	0	0	0
		C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.54	16.61	1.1	C
C-A				
C-B	0.29	10.24	0.4	B
A-B				
A-C				

Main Results for each time segment**08:00 - 08:15**

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	19.00	126.27	0.150	18.82	0.2	8.363	A
C-A	77.00			77.00			
C-B	23.00	123.18	0.187	22.77	0.2	8.944	A
A-B	3.00			3.00			
A-C	122.00			122.00			

08:15 - 08:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	51.00	128.52	0.397	50.53	0.6	11.473	B
C-A	84.00			84.00			
C-B	35.00	122.52	0.286	34.83	0.4	10.244	B
A-B	5.00			5.00			
A-C	123.00			123.00			

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	63.00	116.19	0.542	62.50	1.1	16.610	C
C-A	156.00			156.00			
C-B	26.00	114.37	0.227	26.10	0.3	10.206	B
A-B	7.00			7.00			
A-C	158.00			158.00			

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	32.00	124.35	0.257	32.79	0.4	9.912	A
C-A	109.00			109.00			
C-B	15.00	115.03	0.130	15.15	0.2	9.025	A
A-B	15.00			15.00			
A-C	147.00			147.00			

2023, PM**Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network**Junctions**

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		1.12	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	116	Stream B-AC

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2023	PM	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:00 - 16:15

		To		
		A	B	C
From	A	0.00	4.00	131.00
	B	0.00	0.00	14.00
	C	156.00	23.00	0.00

Demand (PCU/TS)

16:15 - 16:30

		To		
		A	B	C
From	A	0.00	0.00	112.00
	B	0.00	0.00	14.00
	C	145.00	26.00	0.00

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	1.00	106.00
	B	1.00	0.00	13.00
	C	139.00	21.00	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	2.00	116.00
	B	2.00	0.00	15.00
	C	137.00	23.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

From	To		
	A	B	C
A	0	0	0
B	0	0	0
C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.14	8.37	0.2	A
C-A				
C-B	0.21	9.14	0.3	A
A-B				
A-C				

Main Results for each time segment

16:00 - 16:15

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	14.00	132.09	0.106	13.88	0.1	7.605	A
C-A	156.00			156.00			
C-B	23.00	120.98	0.190	22.77	0.2	9.144	A
A-B	4.00			4.00			
A-C	131.00			131.00			

16:15 - 16:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	14.00	137.02	0.102	14.00	0.1	7.315	A
C-A	145.00			145.00			
C-B	26.00	126.05	0.206	25.97	0.3	8.991	A
A-B	0.00			0.00			
A-C	112.00			112.00			

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	14.00	131.40	0.107	14.00	0.1	7.665	A
C-A	139.00			139.00			
C-B	21.00	127.15	0.165	21.06	0.2	8.487	A
A-B	1.00			1.00			
A-C	106.00			106.00			

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	17.00	124.52	0.137	16.96	0.2	8.365	A
C-A	137.00			137.00			
C-B	23.00	124.72	0.184	22.98	0.2	8.843	A
A-B	2.00			2.00			
A-C	116.00			116.00			

Junctions 9
PICADY 9 - Priority Intersection Module
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Filename: Howth Rd Church Rd 2038.j9
Path: C:\Users\Martin.Rogers\Documents\howth road
Report generation date: 11/05/2020 20:03:06

»2038 , AM
 »2038, PM

Summary of junction performance

	AM					PM				
	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity
2038										
Stream B-AC	1.9	24.04	0.68	C	10 %	0.2	8.85	0.16	A	92 %
Stream C-B	0.5	11.44	0.34	B	[Stream B-AC]	0.3	9.94	0.24	A	[Stream B-AC]

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle. Network Residual Capacity indicates the amount by which network flow could be increased before a user-definable threshold (see Analysis Options) is met.

File summary

File Description

Title	Howth SHD
Location	Howth Road Dublin 13
Site number	
Date	11/05/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Hour	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	Residual capacity criteria type	RFC Threshold	Average Delay threshold (s)	Queue threshold (PCU)
	✓	Delay	0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2038	AM	DIRECT	08:00	09:00	60	15
D2	2038	PM	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2038 , AM**Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network**Junctions**

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		4.10	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	10	Stream B-AC

Arms**Arms**

Arm	Name	Description	Arm type
A	Howth Road East		Major
B	Church Road		Minor
C	Howth Road West		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Width for right turn (m)	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
C	7.30		✓	2.20	50.0		-

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
B	One lane	3.00	50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/TS)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	129.627	0.089	0.225	0.142	0.322
1	B-C	163.853	0.095	0.240	-	-
1	C-B	150.730	0.220	0.220	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.
Streams may be combined, in which case capacity will be adjusted.
Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2038	AM	DIRECT	08:00	09:00	60	15

Vehicle mix varies over time	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

		To			
		A	B	C	
08:00 - 08:15	From	A	0.00	4.00	136.00
		B	2.00	0.00	19.00
		C	88.00	27.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	
08:15 - 08:30	From	A	0.00	6.00	137.00
		B	4.00	0.00	56.00
		C	95.00	41.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	
08:30 - 08:45	From	A	0.00	8.00	178.00
		B	6.00	0.00	68.00
		C	179.00	30.00	0.00

Demand (PCU/TS)

08:45 - 09:00	From	To			
			A	B	C
		A	0.00	17.00	165.00
		B	1.00	0.00	36.00
	C	125.00	17.00	0.00	

Vehicle Mix

Heavy Vehicle Percentages

08:00 - 08:15	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:15 - 08:30	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:30 - 08:45	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:45 - 09:00	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.68	24.04	1.9	C
C-A				
C-B	0.34	11.44	0.5	B
A-B				
A-C				

Main Results for each time segment**08:00 - 08:15**

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	21.00	122.83	0.171	20.80	0.2	8.803	A
C-A	88.00			88.00			
C-B	27.00	119.88	0.225	26.71	0.3	9.631	A
A-B	4.00			4.00			
A-C	136.00			136.00			

08:15 - 08:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	60.00	123.65	0.485	59.29	0.9	13.837	B
C-A	95.00			95.00			
C-B	41.00	119.21	0.344	40.77	0.5	11.441	B
A-B	6.00			6.00			
A-C	137.00			137.00			

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	74.00	109.43	0.676	72.98	1.9	24.036	C
C-A	179.00			179.00			
C-B	30.00	109.74	0.273	30.13	0.4	11.326	B
A-B	8.00			8.00			
A-C	178.00			178.00			

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	37.00	119.81	0.309	38.48	0.5	11.257	B
C-A	125.00			125.00			
C-B	17.00	110.62	0.154	17.20	0.2	9.653	A
A-B	17.00			17.00			
A-C	165.00			165.00			

2038, PM**Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network**Junctions**

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		1.22	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	92	Stream B-AC

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2038	PM	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:00 - 16:15

		To		
		A	B	C
From	A	0.00	4.00	149.00
	B	0.00	0.00	16.00
	C	176.00	27.00	0.00

Demand (PCU/TS)

16:15 - 16:30

		To		
		A	B	C
From	A	0.00	0.00	127.00
	B	0.00	0.00	16.00
	C	163.00	30.00	0.00

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	1.00	120.00
	B	1.00	0.00	14.00
	C	156.00	24.00	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	2.00	132.00
	B	2.00	0.00	17.00
	C	153.00	27.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

From	To		
	A	B	C
A	0	0	0
B	0	0	0
C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.16	8.85	0.2	A
C-A				
C-B	0.24	9.94	0.3	A
A-B				
A-C				

Main Results for each time segment

16:00 - 16:15

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	16.00	127.78	0.125	15.86	0.1	8.032	A
C-A	176.00			176.00			
C-B	27.00	117.01	0.231	26.70	0.3	9.935	A
A-B	4.00			4.00			
A-C	149.00			149.00			

16:15 - 16:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	16.00	133.43	0.120	16.00	0.1	7.664	A
C-A	163.00			163.00			
C-B	30.00	122.74	0.244	29.98	0.3	9.700	A
A-B	0.00			0.00			
A-C	127.00			127.00			

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	15.00	127.75	0.117	15.00	0.1	7.984	A
C-A	156.00			156.00			
C-B	24.00	124.06	0.193	24.08	0.2	9.008	A
A-B	1.00			1.00			
A-C	120.00			120.00			

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	19.00	120.57	0.158	18.95	0.2	8.851	A
C-A	153.00			153.00			
C-B	27.00	121.20	0.223	26.96	0.3	9.546	A
A-B	2.00			2.00			
A-C	132.00			132.00			

APPENDIX 4D

HOWTH ROAD / OFFINGTON PARK PRIORITY JUNCTION

Junctions 9
PICADY 9 - Priority Intersection Module
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Filename: Howth Rd Offington Pk 2023.j9
Path: C:\Users\Martin.Rogers\Documents\howth road
Report generation date: 11/05/2020 20:23:41

»2023 , AM
 »2023, PM

Summary of junction performance

	AM					PM				
	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity
2023										
Stream B-AC	1.4	23.80	0.60	C	12 %	0.5	14.70	0.33	B	47 %
Stream C-B	0.3	10.11	0.23	B	[Stream B-AC]	0.2	8.83	0.16	A	[Stream B-AC]

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle. Network Residual Capacity indicates the amount by which network flow could be increased before a user-definable threshold (see Analysis Options) is met.

File summary

File Description

Title	Howth SHD
Location	Howth Road Dublin 13
Site number	
Date	11/05/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Hour	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	Residual capacity criteria type	RFC Threshold	Average Delay threshold (s)	Queue threshold (PCU)
	✓	Delay	0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM	DIRECT	08:00	09:00	60	15
D2	2023	PM	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2023 , AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		4.08	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	12	Stream B-AC

Arms

Arms

Arm	Name	Description	Arm type
A	Howth Road East		Major
B	Offington Park		Minor
C	Howth Road West		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Width for right turn (m)	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
C	7.30		✓	2.20	50.0		-

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
B	One lane	3.00	50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/TS)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	129.627	0.089	0.225	0.142	0.322
1	B-C	163.853	0.095	0.240	-	-
1	C-B	150.730	0.220	0.220	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.
Streams may be combined, in which case capacity will be adjusted.
Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM	DIRECT	08:00	09:00	60	15

Vehicle mix varies over time	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

		To			
		A	B	C	
08:00 - 08:15	From	A	0.00	15.00	108.00
		B	26.00	0.00	18.00
		C	61.00	3.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	
08:15 - 08:30	From	A	0.00	16.00	116.00
		B	14.00	0.00	14.00
		C	74.00	7.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	
08:30 - 08:45	From	A	0.00	27.00	137.00
		B	29.00	0.00	25.00
		C	118.00	26.00	0.00

Demand (PCU/TS)

08:45 - 09:00	From	To			
			A	B	C
		A	0.00	14.00	128.00
		B	28.00	0.00	18.00
	C	110.00	26.00	0.00	

Vehicle Mix

Heavy Vehicle Percentages

08:00 - 08:15	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:15 - 08:30	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:30 - 08:45	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:45 - 09:00	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.60	23.80	1.4	C
C-A				
C-B	0.23	10.11	0.3	B
A-B				
A-C				

Main Results for each time segment**08:00 - 08:15**

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	44.00	108.01	0.407	43.33	0.7	13.778	B
C-A	61.00			61.00			
C-B	3.00	123.62	0.024	2.98	0.0	7.457	A
A-B	15.00			15.00			
A-C	108.00			108.00			

08:15 - 08:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	28.00	107.28	0.261	28.31	0.4	11.439	B
C-A	74.00			74.00			
C-B	7.00	121.64	0.058	6.96	0.1	7.845	A
A-B	16.00			16.00			
A-C	116.00			116.00			

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	54.00	89.78	0.601	52.94	1.4	23.801	C
C-A	118.00			118.00			
C-B	26.00	114.59	0.227	25.77	0.3	10.106	B
A-B	27.00			27.00			
A-C	137.00			137.00			

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	46.00	90.77	0.507	46.35	1.1	20.426	C
C-A	110.00			110.00			
C-B	26.00	119.43	0.218	26.01	0.3	9.636	A
A-B	14.00			14.00			
A-C	128.00			128.00			

2023, PM**Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network**Junctions**

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		1.62	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	47	Stream B-AC

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2023	PM	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:00 - 16:15

		To		
		A	B	C
From	A	0.00	10.00	114.00
	B	14.00	0.00	8.00
	C	134.00	17.00	0.00

Demand (PCU/TS)

16:15 - 16:30

		To		
		A	B	C
From	A	0.00	9.00	103.00
	B	14.00	0.00	11.00
	C	125.00	9.00	0.00

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	15.00	101.00
	B	7.00	0.00	6.00
	C	129.00	13.00	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	21.00	111.00
	B	21.00	0.00	9.00
	C	120.00	20.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

From	To		
	A	B	C
A	0	0	0
B	0	0	0
C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.33	14.70	0.5	B
C-A				
C-B	0.16	8.83	0.2	A
A-B				
A-C				

Main Results for each time segment

16:00 - 16:15

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	22.00	92.79	0.237	21.69	0.3	12.606	B
C-A	134.00			134.00			
C-B	17.00	123.40	0.138	16.84	0.2	8.433	A
A-B	10.00			10.00			
A-C	114.00			114.00			

16:15 - 16:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	25.00	102.30	0.244	24.99	0.3	11.638	B
C-A	125.00			125.00			
C-B	9.00	126.05	0.071	9.08	0.1	7.699	A
A-B	9.00			9.00			
A-C	103.00			103.00			

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	13.00	101.79	0.128	13.17	0.1	10.174	B
C-A	129.00			129.00			
C-B	13.00	125.16	0.104	12.96	0.1	8.018	A
A-B	15.00			15.00			
A-C	101.00			101.00			

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	30.00	90.56	0.331	29.66	0.5	14.701	B
C-A	120.00			120.00			
C-B	20.00	121.64	0.164	19.92	0.2	8.834	A
A-B	21.00			21.00			
A-C	111.00			111.00			

<h1>Junctions 9</h1>
<h2>PICADY 9 - Priority Intersection Module</h2>
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Filename: Howth Rd Offington Pk 2038.j9
Path: C:\Users\Martin.Rogers\Documents\howth road
Report generation date: 11/05/2020 20:16:44

»2038 , AM
 »2038, PM

Summary of junction performance

	AM					PM				
	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity
2038										
Stream B-AC	2.8	38.79	0.77	E	-2 %	0.7	17.80	0.41	C	29 %
Stream C-B	0.4	11.14	0.27	B	[Stream B-AC]	0.2	9.48	0.20	A	[Stream B-AC]

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle. Network Residual Capacity indicates the amount by which network flow could be increased before a user-definable threshold (see Analysis Options) is met.

File summary

File Description

Title	Howth SHD
Location	Howth Road Dublin 13
Site number	
Date	11/05/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Hour	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	Residual capacity criteria type	RFC Threshold	Average Delay threshold (s)	Queue threshold (PCU)
	✓	Delay	0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2038	AM	DIRECT	08:00	09:00	60	15
D2	2038	PM	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2038 , AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		6.52	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	-2	Stream B-AC

Arms

Arms

Arm	Name	Description	Arm type
A	Howth Road East		Major
B	Offington Park		Minor
C	Howth Road West		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Width for right turn (m)	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
C	7.30		✓	2.20	50.0		-

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
B	One lane	3.00	50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/TS)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	129.627	0.089	0.225	0.142	0.322
1	B-C	163.853	0.095	0.240	-	-
1	C-B	150.730	0.220	0.220	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments. Streams may be combined, in which case capacity will be adjusted. Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2038	AM	DIRECT	08:00	09:00	60	15

Vehicle mix varies over time	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:00 - 08:15

		To		
		A	B	C
From	A	0.00	17.00	120.00
	B	30.00	0.00	21.00
	C	69.00	4.00	0.00

Demand (PCU/TS)

08:15 - 08:30

		To		
		A	B	C
From	A	0.00	18.00	130.00
	B	16.00	0.00	16.00
	C	84.00	8.00	0.00

Demand (PCU/TS)

08:30 - 08:45

		To		
		A	B	C
From	A	0.00	31.00	153.00
	B	34.00	0.00	29.00
	C	135.00	30.00	0.00

Demand (PCU/TS)

08:45 - 09:00	From	To			
			A	B	C
		A	0.00	16.00	143.00
		B	33.00	0.00	21.00
	C	125.00	30.00	0.00	

Vehicle Mix

Heavy Vehicle Percentages

08:00 - 08:15	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:15 - 08:30	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:30 - 08:45	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Heavy Vehicle Percentages

08:45 - 09:00	From	To			
			A	B	C
		A	0	0	0
		B	0	0	0
	C	0	0	0	

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.77	38.79	2.8	E
C-A				
C-B	0.27	11.14	0.4	B
A-B				
A-C				

Main Results for each time segment**08:00 - 08:15**

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	51.00	103.96	0.491	50.07	0.9	16.422	C
C-A	69.00			69.00			
C-B	4.00	120.54	0.033	3.97	0.0	7.719	A
A-B	17.00			17.00			
A-C	120.00			120.00			

08:15 - 08:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	32.00	102.45	0.312	32.47	0.5	12.945	B
C-A	84.00			84.00			
C-B	8.00	118.11	0.068	7.96	0.1	8.168	A
A-B	18.00			18.00			
A-C	130.00			130.00			

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	63.00	82.06	0.768	60.70	2.8	38.786	E
C-A	135.00			135.00			
C-B	30.00	110.18	0.272	29.70	0.4	11.143	B
A-B	31.00			31.00			
A-C	153.00			153.00			

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	54.00	83.81	0.644	54.81	2.0	31.895	D
C-A	125.00			125.00			
C-B	30.00	115.69	0.259	30.01	0.4	10.506	B
A-B	16.00			16.00			
A-C	143.00			143.00			

2038, PM**Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network**Junctions**

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		1.94	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	29	Stream B-AC

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2038	PM	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:00 - 16:15

		To		
		A	B	C
From	A	0.00	12.00	130.00
	B	16.00	0.00	10.00
	C	151.00	19.00	0.00

Demand (PCU/TS)

16:15 - 16:30

		To		
		A	B	C
From	A	0.00	11.00	117.00
	B	16.00	0.00	13.00
	C	139.00	11.00	0.00

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	17.00	114.00
	B	8.00	0.00	7.00
	C	145.00	14.00	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	24.00	126.00
	B	24.00	0.00	11.00
	C	134.00	23.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

From	To		
	A	B	C
A	0	0	0
B	0	0	0
C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.41	17.80	0.7	C
C-A				
C-B	0.20	9.48	0.2	A
A-B				
A-C				

Main Results for each time segment

16:00 - 16:15

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	26.00	86.98	0.299	25.58	0.4	14.565	B
C-A	151.00			151.00			
C-B	19.00	119.43	0.159	18.81	0.2	8.929	A
A-B	12.00			12.00			
A-C	130.00			130.00			

16:15 - 16:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	29.00	96.95	0.299	28.99	0.4	13.240	B
C-A	139.00			139.00			
C-B	11.00	122.52	0.090	11.09	0.1	8.084	A
A-B	11.00			11.00			
A-C	117.00			117.00			

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	15.00	96.57	0.155	15.24	0.2	11.098	B
C-A	145.00			145.00			
C-B	14.00	121.86	0.115	13.97	0.1	8.340	A
A-B	17.00			17.00			
A-C	114.00			114.00			

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	35.00	84.60	0.414	34.50	0.7	17.798	C
C-A	134.00			134.00			
C-B	23.00	117.67	0.195	22.89	0.2	9.481	A
A-B	24.00			24.00			
A-C	126.00			126.00			

APPENDIX 4E

HARBOUR ROAD / CHURCH STREET PRIORITY JUNCTION

Junctions 9
PICADY 9 - Priority Intersection Module
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Filename: Harbour Rd Church St 2023.j9
Path: C:\Users\Martin.Rogers\Documents\howth road
Report generation date: 11/05/2020 20:38:17

»2023 , AM
 »2023, PM

Summary of junction performance

	AM					PM				
	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity
2023										
Stream B-AC	0.4	9.56	0.30	A	56 %	0.2	8.07	0.20	A	118 %
Stream C-B	0.8	13.00	0.44	B	[Stream C-B]	0.4	9.76	0.31	A	[Stream C-B]

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle. Network Residual Capacity indicates the amount by which network flow could be increased before a user-definable threshold (see Analysis Options) is met.

File summary

File Description

Title	Howth SHD
Location	Howth Road Dublin 13
Site number	
Date	11/05/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Hour	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	Residual capacity criteria type	RFC Threshold	Average Delay threshold (s)	Queue threshold (PCU)
	✓	Delay	0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM	DIRECT	08:00	09:00	60	15
D2	2023	PM	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2023 , AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		3.72	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	56	Stream C-B

Arms

Arms

Arm	Name	Description	Arm type
A	Harbour Road East		Major
B	Church Street		Minor
C	Harbour Road West		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Width for right turn (m)	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
C	7.30		✓	2.20	50.0		-

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
B	One lane	3.00	50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/TS)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	129.627	0.089	0.225	0.142	0.322
1	B-C	163.853	0.095	0.240	-	-
1	C-B	150.730	0.220	0.220	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments. Streams may be combined, in which case capacity will be adjusted. Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM	DIRECT	08:00	09:00	60	15

Vehicle mix varies over time	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

		To			
		A	B	C	
08:00 - 08:15	From	A	0.00	0.00	94.00
		B	1.00	0.00	30.00
		C	41.00	24.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	
08:15 - 08:30	From	A	0.00	1.00	88.00
		B	2.00	0.00	29.00
		C	41.00	28.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	
08:30 - 08:45	From	A	0.00	2.00	110.00
		B	1.00	0.00	40.00
		C	58.00	53.00	0.00

Demand (PCU/TS)

08:45 - 09:00		To			
		A	B	C	
	From	A	0.00	2.00	119.00
	B	0.00	0.00	38.00	
	C	65.00	55.00	0.00	

Vehicle Mix

Heavy Vehicle Percentages

08:00 - 08:15		To			
		A	B	C	
	From	A	0	0	0
	B	0	0	0	
	C	0	0	0	

Heavy Vehicle Percentages

08:15 - 08:30		To			
		A	B	C	
	From	A	0	0	0
	B	0	0	0	
	C	0	0	0	

Heavy Vehicle Percentages

08:30 - 08:45		To			
		A	B	C	
	From	A	0	0	0
	B	0	0	0	
	C	0	0	0	

Heavy Vehicle Percentages

08:45 - 09:00		To			
		A	B	C	
	From	A	0	0	0
	B	0	0	0	
	C	0	0	0	

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.30	9.56	0.4	A
C-A				
C-B	0.44	13.00	0.8	B
A-B				
A-C				

Main Results for each time segment**08:00 - 08:15**

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	31.00	139.14	0.223	30.72	0.3	8.280	A
C-A	41.00			41.00			
C-B	24.00	130.01	0.185	23.78	0.2	8.454	A
A-B	0.00			0.00			
A-C	94.00			94.00			

08:15 - 08:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	31.00	138.20	0.224	31.00	0.3	8.395	A
C-A	41.00			41.00			
C-B	28.00	131.11	0.214	27.96	0.3	8.720	A
A-B	1.00			1.00			
A-C	88.00			88.00			

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	41.00	134.88	0.304	40.86	0.4	9.558	A
C-A	58.00			58.00			
C-B	53.00	126.05	0.420	52.56	0.7	12.175	B
A-B	2.00			2.00			
A-C	110.00			110.00			

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	38.00	135.12	0.281	38.03	0.4	9.275	A
C-A	65.00			65.00			
C-B	55.00	124.06	0.443	54.93	0.8	13.000	B
A-B	2.00			2.00			
A-C	119.00			119.00			

2023, PM**Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network**Junctions**

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		2.26	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	118	Stream C-B

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2023	PM	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:00 - 16:15

		To		
		A	B	C
From	A	0.00	3.00	74.00
	B	1.00	0.00	22.00
	C	81.00	31.00	0.00

Demand (PCU/TS)

16:15 - 16:30

		To		
		A	B	C
From	A	0.00	0.00	92.00
	B	0.00	0.00	17.00
	C	90.00	35.00	0.00

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	3.00	73.00
	B	1.00	0.00	18.00
	C	98.00	42.00	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	1.00	88.00
	B	1.00	0.00	27.00
	C	94.00	35.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

From	To		
	A	B	C
A	0	0	0
B	0	0	0
C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.20	8.07	0.2	A
C-A				
C-B	0.31	9.76	0.4	A
A-B				
A-C				

Main Results for each time segment

16:00 - 16:15

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	23.00	142.14	0.162	22.81	0.2	7.529	A
C-A	81.00			81.00			
C-B	31.00	133.76	0.232	30.70	0.3	8.708	A
A-B	3.00			3.00			
A-C	74.00			74.00			

16:15 - 16:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	17.00	141.77	0.120	17.05	0.1	7.218	A
C-A	90.00			90.00			
C-B	35.00	130.45	0.268	34.94	0.4	9.415	A
A-B	0.00			0.00			
A-C	92.00			92.00			

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	19.00	140.85	0.135	18.98	0.2	7.385	A
C-A	98.00			98.00			
C-B	42.00	133.98	0.313	41.91	0.4	9.765	A
A-B	3.00			3.00			
A-C	73.00			73.00			

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	28.00	139.29	0.201	27.91	0.2	8.073	A
C-A	94.00			94.00			
C-B	35.00	131.11	0.267	35.08	0.4	9.379	A
A-B	1.00			1.00			
A-C	88.00			88.00			

Junctions 9
PICADY 9 - Priority Intersection Module
For sales and distribution information, program advice and maintenance, contact TRL: +44 (0)1344 379777 software@trl.co.uk www.trlsoftware.co.uk
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Filename: Harbour Rd Church St 2038.j9
Path: C:\Users\Martin.Rogers\Documents\howth road
Report generation date: 11/05/2020 20:47:28

»2023 , AM
 »2023, PM

Summary of junction performance

	AM					PM				
	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity	Queue (PCU)	Delay (s)	RFC	LOS	Network Residual Capacity
2023										
Stream B-AC	0.5	10.57	0.36	B	37 %	0.3	8.57	0.23	A	92 %
Stream C-B	1.1	15.47	0.52	C	[Stream C-B]	0.6	10.68	0.36	B	[Stream C-B]

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle. Network Residual Capacity indicates the amount by which network flow could be increased before a user-definable threshold (see Analysis Options) is met.

File summary

File Description

Title	Howth SHD
Location	Howth Road Dublin 13
Site number	
Date	11/05/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ICTDOMAIN\martin.rogers
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Hour	perMin

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	Residual capacity criteria type	RFC Threshold	Average Delay threshold (s)	Queue threshold (PCU)
	✓	Delay	0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM	DIRECT	08:00	09:00	60	15
D2	2023	PM	DIRECT	16:00	17:00	60	15

Analysis Set Details

ID	Network flow scaling factor (%)
A1	100.000

2023 , AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		4.41	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	37	Stream C-B

Arms

Arms

Arm	Name	Description	Arm type
A	Harbour Road East		Major
B	Church Street		Minor
C	Harbour Road West		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Width for right turn (m)	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
C	7.30		✓	2.20	50.0		-

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
B	One lane	3.00	50	50

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/TS)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	129.627	0.089	0.225	0.142	0.322
1	B-C	163.853	0.095	0.240	-	-
1	C-B	150.730	0.220	0.220	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments. Streams may be combined, in which case capacity will be adjusted. Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D1	2023	AM	DIRECT	08:00	09:00	60	15

Vehicle mix varies over time	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

		To			
		A	B	C	
08:00 - 08:15	From	A	0.00	0.00	104.00
		B	1.00	0.00	35.00
		C	45.00	28.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	
08:15 - 08:30	From	A	0.00	1.00	97.00
		B	2.00	0.00	33.00
		C	46.00	33.00	0.00

Demand (PCU/TS)

		To			
		A	B	C	
08:30 - 08:45	From	A	0.00	2.00	123.00
		B	1.00	0.00	46.00
		C	65.00	62.00	0.00

Demand (PCU/TS)

08:45 - 09:00		To			
		A	B	C	
	From	A	0.00	2.00	133.00
	B	0.00	0.00	43.00	
C	73.00	63.00	0.00		

Vehicle Mix

Heavy Vehicle Percentages

08:00 - 08:15		To			
		A	B	C	
	From	A	0	0	0
	B	0	0	0	
C	0	0	0		

Heavy Vehicle Percentages

08:15 - 08:30		To			
		A	B	C	
	From	A	0	0	0
	B	0	0	0	
C	0	0	0		

Heavy Vehicle Percentages

08:30 - 08:45		To			
		A	B	C	
	From	A	0	0	0
	B	0	0	0	
C	0	0	0		

Heavy Vehicle Percentages

08:45 - 09:00		To			
		A	B	C	
	From	A	0	0	0
	B	0	0	0	
C	0	0	0		

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.36	10.57	0.5	B
C-A				
C-B	0.52	15.47	1.1	C
A-B				
A-C				

Main Results for each time segment**08:00 - 08:15**

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	36.00	136.92	0.263	35.65	0.4	8.856	A
C-A	45.00			45.00			
C-B	28.00	127.81	0.219	27.72	0.3	8.969	A
A-B	0.00			0.00			
A-C	104.00			104.00			

08:15 - 08:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	35.00	136.23	0.257	35.00	0.3	8.890	A
C-A	46.00			46.00			
C-B	33.00	129.13	0.256	32.94	0.3	9.350	A
A-B	1.00			1.00			
A-C	97.00			97.00			

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	47.00	131.78	0.357	46.80	0.5	10.567	B
C-A	65.00			65.00			
C-B	62.00	123.18	0.503	61.36	1.0	14.413	B
A-B	2.00			2.00			
A-C	123.00			123.00			

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	43.00	131.77	0.326	43.05	0.5	10.151	B
C-A	73.00			73.00			
C-B	63.00	120.98	0.521	62.92	1.1	15.471	C
A-B	2.00			2.00			
A-C	133.00			133.00			

2023, PM**Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network**Junctions**

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	Site 2	T-Junction	Two-way		2.54	A

Junction Network Options

Driving side	Lighting	Network residual capacity (%)	First arm reaching threshold
Left	Normal/unknown	92	Stream C-B

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)
D2	2023	PM	DIRECT	16:00	17:00	60	15

Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Scaling Factor (%)
A		✓	100.000
B		✓	100.000
C		✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:00 - 16:15

		To		
		A	B	C
From	A	0.00	4.00	83.00
	B	1.00	0.00	22.00
	C	81.00	31.00	0.00

Demand (PCU/TS)

16:15 - 16:30

		To		
		A	B	C
From	A	0.00	0.00	92.00
	B	0.00	0.00	26.00
	C	99.00	41.00	0.00

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	4.00	81.00
	B	1.00	0.00	21.00
	C	108.00	48.00	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	1.00	99.00
	B	1.00	0.00	31.00
	C	104.00	40.00	0.00

Vehicle Mix

Heavy Vehicle Percentages

	To			
	A	B	C	
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS
B-AC	0.23	8.57	0.3	A
C-A				
C-B	0.36	10.68	0.6	B
A-B				
A-C				

Main Results for each time segment

16:00 - 16:15

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	23.00	139.87	0.164	22.81	0.2	7.676	A
C-A	81.00			81.00			
C-B	31.00	131.56	0.236	30.70	0.3	8.897	A
A-B	4.00			4.00			
A-C	83.00			83.00			

16:15 - 16:30

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	26.00	141.78	0.183	25.97	0.2	7.770	A
C-A	99.00			99.00			
C-B	41.00	130.45	0.314	40.85	0.5	10.027	B
A-B	0.00			0.00			
A-C	92.00			92.00			

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	22.00	139.08	0.158	22.03	0.2	7.691	A
C-A	108.00			108.00			
C-B	48.00	132.00	0.364	47.89	0.6	10.684	B
A-B	4.00			4.00			
A-C	81.00			81.00			

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	32.00	136.77	0.234	31.89	0.3	8.571	A
C-A	104.00			104.00			
C-B	40.00	128.69	0.311	40.10	0.5	10.173	B
A-B	1.00			1.00			
A-C	99.00			99.00			